CITY OF BLOOMINGTON



June 10, 2024 @ 4:00 p.m.

401 N. Morton Street Kelly Conference Room #155 & via Zoom:

https://bloomington.zoom.us/j/86714253039?pwd=SXJ2bmNwRFhLeVZSRW44TVI0T3hZUT09

Meeting ID: 867 1425 3039 Passcode: 064896

CITY OF BLOOMINGTON PLAT COMMITTEE
June 10, 2024 at 4:00 p.m.

401 N. Morton Street, City Hall Kelly Conference Room #155

HYBRID MEETING:

https://bloomington.zoom.us/j/86714253039?pwd=SXJ2bmNwRFhLeVZSRW44TVI 0T3hZUT09

Meeting ID: 867 1425 3039 Password: 064896

PETITION MAP: https://arcg.is/1WeP8m

ROLL CALL

MINUTES TO BE APPROVED:

REPORTS, RESOLUTIONS, AND COMMUNICATIONS:

PETITIONS:

DP-23-24/PLAT2024-05-0029 City of Bloomington Engineering Dept.

501,601,707,711, and 719 W 2nd Street

Secondary plat approval to create new lots and new Right-of-way in the Hopewell neighborhood.

Updated: 6/7/2024

Case Manager: Gabriel Holbrow

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Auxiliary aids for people with disabilities are available upon request with adequate notice. Please call <u>812-349-3429</u> or e-mail <u>human.rights@bloomington.in.gov</u>.

^{**}Next Meeting Date: July 15, 2024

CASE #: DP-23-24 / PLAT2024-05-0029

DATE: June 10, 2024

BLOOMINGTON PLAT COMMITTEE STAFF REPORT

Location: West of South Rogers Street between West 1st Street and West 2nd Street (addresses of current parcels: 501, 601, 707, and 711 West 2nd Street)

PETITIONER: City of Bloomington Engineering Department

401 North Morton Street

Bloomington, IN

OWNER: Bloomington Redevelopment Commission

401 North Morton Street, Suite 130

Bloomington, IN

CONSULTANTS: Crossroad Engineers

115 North 17th Avenue

Beech Grove, IN

REQUEST: The petitioner is requesting secondary plat approval to create new lots and new right-of-way in the Hopewell neighborhood.

BACKGROUND:

Area: 11.33 acres

Current Zoning: Mixed-Use Medium Scale (MM) within the Transform Redevelopment

Overlay (TRO)

Comprehensive

Plan Designation: Mixed Urban Residential / Former Bloomington Hospital Focus Area

Existing Land Use: Vacant (former hospital site)

Proposed Land Use: No change proposed

Surrounding Uses: North – Park; Office; Medical Clinic; Personal Services

South – Vacant buildings (former hospital site) East – Vacant land; Office; Medical Clinic

West - Vacant buildings; Office

REPORT: The property is located west of South Rogers Street between West 1st Street and West 2nd Street. The petition site is a portion of the former site of the IU Health Bloomington Hospital. Development of the larger area is guided by the Bloomington Hospital Site Redevelopment Master Plan (Master Plan), released in January 2021. The petition site is occasionally called Hopewell West, part of the future Hopewell neighborhood. The property is owned by the City of Bloomington Redevelopment Commission (RDC).

The entire petition site is located in the Mixed-Use Medium Scale (MM) zoning district within the Transform Redevelopment Overlay (TRO). Adjacent properties to the north across West 2nd Street include Building and Trades Park, zoned Parks and Open Space (PO), as well as several properties zoned MM within the TRO which contain office, medical clinic, and personal services uses. To the south across West 1st Street, the property at the southwest corner of Rogers and 1st is zoned Residential Multifamily (RM) within the TRO, while the properties immediately west of that are zoned Residential Urban Lot (R4) and are not within the TRO. The properties to the south contain vacant buildings or recently demolished buildings that were formally used as part of the hospital

site. To the east across South Rogers Street is the area occasionally known as Hopewell Phase I East which contains vacant land currently under development and is zoned MM within the TRO, as well as the Centerstone building and property which is zoned Mixed-Use Neighborhood Scale (MN) within the TRO. Adjacent properties to the west are zoned MM and Mixed-Use Institutional (MI) within the TRO and contain vacant buildings and office space owned by the Monroe County Community School Corporation (MCCSC), as well as one privately owned office building.

The petitioner is requesting secondary plat approval to reconfigure the existing lots and rights-of-way in this portion of the Master Plan area to re-create an urban pattern of streets, alleys, and blocks to facilitate redevelopment. The plat creates new public right-of-way for two new north-south streets, South Fairview Street and South Jackson Street; an east-west greenway street bordered by green space and park amenities, West University Street; as well as five alleys. The plat contains 19 lots, including lots for the two existing buildings that remain: a parking garage in the center north of the site and the Kohr building in the southeast corner of the site. The Kohr building, formerly part of the hospital complex, has been locally designated as a one-building historic district and is intended to be redeveloped as affordable housing. Lots 7, 11, 12, and 16 along West University Street are intended to be greenspace lots managed by the City of Bloomington Parks and Recreation Department as public park space containing park amenities as well as stormwater drainage facilities for the neighborhood.

The primary plat was approved by the Plan Commission as DP-23-23 on July 10, 2023 with five conditions, all of which have been met as explained below.

Primary Plat condition 1: Right-of-way vacation for the existing alleys on the petition site shall be approved by City of Bloomington Common Council prior to secondary plat recording.

• City Council approved vacation of the existing alleys as Ordinance 23-22 on October 4, 2023.

Primary Plat condition 2: Greenspace lots intended for public use that are labeled as "common area 2" through "common area 5" on the primary plat shall be correctly labeled as lots on the secondary plat.

• On the secondary plat, these lots are labeled lot 3, lot 7, lot 11, lot 12, and lot 16.

Primary Plat condition 3: The secondary plat shall provide public right-of-way for West University Street west of South Fairview Street, NC-45 in the Transportation Plan, to provide the opportunity for a future street connection to the west. The 332 foot long portion of the proposed common area 1 parcel shall be included in the Fairview Street right-of-way. The northern portion may need to be absorbed into Lot 2.

- The secondary plat dedicates right-of-way west of Fairview Street within the continuation
 of the alignment of West University Street. Although this continuation right-of-way is not
 wide enough be a street on its own, it could be expanded into full street width for a
 continuation of West University Street if the property to the west were ever subdivided or
 acquired by the City.
- The right-of-way of Fairview Street south of West University Street has been widened to the west 15 feet, all the way to west edge of the RDC-owned property, to avoid creating an unbuildable sliver of a lot. Only the eastern 60 feet of this right-of-way is proposed to be constructed as a street. The remaining 15 feet will be a graded slope to up from the constructed Fairview Street to the existing grade of the property to the west.

• What had been the northern portion of common area 1 on the primary plat has become lot 3 on the secondary plat. It did not need to be absorbed into lot 2.

Primary Plat condition 4: The seven lots labeled 19 through 25 shall be reduced to six lots on the secondary plat so that all six lots have a depth-to-width ratio not exceeding four-to-one.

• What had been lots 18-26 on the primary plat has been combined into one lot, numbered lot 15, on the secondary plat. This lot 15 has a depth-to-width ratio of 0.52 to one, which is less than four-to-one, which complies with the intent of condition 4.

Primary Plat condition 5: The secondary plat shall provide eight additional feet of right-of-way dedication along South Rogers Street between West 2nd Street and West University Street to bring the total right-of-way dedication in this segment to 50 feet from the established apparent centerline of the roadway.

• The secondary plat provides 50 feet of right-of-way to the west of the established apparent centerline of South Rogers Street between West 2nd Street and West University Street.

Typically, in conjunction with the approval of a secondary plat, the subdivision petitioner is required to provide a financial performance guarantee that all public facility improvements and installations shall be completed within two years, with possible extensions for up to one additional year. However, the UDO provides an exception that the posting of a performance guarantee is not required when the petitioner is the City of Bloomington. As the petitioner for this subdivision is the City Engineering Department and the property owner is the Bloomington Redevelopment Commission, both constituent bodies of the City of Bloomington, no performance guarantee is required.

20.06.060(c)(3)(D) SECONDARY PLAT REVIEW AND DECISION: The Plan Commission or Plat Committee shall review the secondary subdivision petition and approve, approve with conditions, or deny the petition in accordance with Section 20.06.040(g) (Review and Decision), based on the general approval criteria in Section 20.06.040(d)(6)(B) (General Compliance Criteria).

20.06.040(d)(6)(B) General Compliance Criteria

- i. Compliance with this UDO
- ii. Compliance with Other Applicable Regulations
- iii. Compliance with Utility, Service, and Improvement Standards
- iv. Compliance with Prior Approvals

PROPOSED FINDING: The plat complies with all of the requirements of the UDO. The plat is compliant with the Transportation Plan. The subdivision follows the guidance of the Bloomington Hospital Site Redevelopment Master Plan. The drainage infrastructure and utilities have been designed in consultation with the City of Bloomington Utilities Department and final approval from the City of Bloomington Utilities Department is required prior to the issuance of any permits to construct the public infrastructure for the subdivision. The secondary plat complies with the primary plat for this subdivision as approved by the Plan Commission on July 10, 2023.

PLAT REVIEW: The proposed subdivision follows the Infill Subdivision (IS) design standards with modifications as required by the Transform Redevelopment Overlay (TRO) in the City of Bloomington Unified Development Ordinance (UDO)

Infill Subdivision Standards as modified by TRO section 20.02.050(b)(11)(A):

Parent tract size: No minimum parent tract size. Maximum parent tract size is three acres. The parent tract is 11.33 acres. The Plan Commission granted a waiver with primary plat approval to allow the parent tract size to exceed the maximum. The larger tract size is a necessary and integral aspect of this unique early stage of the development of the Hopewell neighborhood.

Open space: Not required. The proposal provides four greenspace lots intended to be open space managed by the City of Bloomington Parks and Recreation Department.

Lots served by alleys: Minimum 100 percent. 14 of the 19 proposed lots (73 percent) are served by public alleys. The Plan Commission granted a waiver with primary plat approval to allow five lots along the West University Street greenway to lack alley access. The proposal establishes pedestrian easements to all five lots along the West University Street greenway, providing access from the side opposite the street frontage.

Block length: Maximum 400 feet. All proposed blocks are between 217 feet and 332 feet.

Cul-de-sac length: Not permitted. No culs-de-sac are proposed. The proposal includes three dead-end alleys; however, because alleys are not streets, these are not considered culs-de-sac.

Transportation facilities: Required to meet Transportation Plan guidance. In the Transportation Plan, West 2nd Street and South Rogers Street are designated as the General Urban street typology with 84 feet of right-of-way width, while West 1st Street and the new interior street grid are designated as the Neighborhood Residential street typology with 60 feet of right-of-way width. The proposal provides additional right-of-way dedication along West 2nd Street and South Rogers Street to bring both streets up to the proposed width. The proposal shows West 1st Street, South Fairview Street, and South Jackson Street designed to Neighborhood Residential street guidance with at least 60 feet of right-of-way width. As guided by the Master Plan, the proposed West University Street greenway follows a modified Shared Street typology with a 55-foot right-of-way width.

On-street parking: Per Transportation Plan guidance. Where provided, on-street parking shall comply with City standards. The proposal shows on-street parking on all block faces along South Jackson Street and South Fairview Street as well as on the north side of both blocks of the West University Street greenway. The proposed parking complies with width guidance in the Transportation Plan and complies with other City standards.

Tree plot width: Per Transportation Plan, or seven feet, whichever is greater. The proposal shows greenscapes on both sides of South Fairview Street and South Jackson Street and on the west side of South Rogers Street that vary from five feet, which is the minimum per the Transportation Plan, to 12 feet. Along the West University Street greenway, the proposal shows stormwater garden and activity zones up to 41 ½ feet on the adjacent greenspace

lots. Greenscape facilities in the public right-of-way of West 2nd Street and West 1st Street will be provided by separate City projects to redesign and reconstruct these existing streets. The Plan Commission granted a waiver with primary plat approval to allow the proposed tree plots widths.

Sidewalk/multiuse path width: Per Transportation Plan, or eight feet, whichever is greater. The proposal shows six-foot-wide pedestrian zones within the public right-of-way along both sides of all new streets, supplemented by 15-foot-wide sidewalk/café zones on the greenspace lots adjacent to the West University Street greenway. The proposal shows a ten-foot-wide sidewalk on the west side of South Rogers Street as well as five-foot-wide center-curb-separated bicycle lanes on both sides of South Rogers Street. Pedestrian and bicycle facilities in the public right-of-way of West 2nd Street and West 1st Street are not shown, but will be provided by separate City projects to redesign and reconstruct these existing streets. The Plan Commission granted a waiver with primary plat approval to allow for a minimum of six feet in width for pedestrian facilities on the new streets.

Lot Establishment Standards:

Lot area and lot width: There is no minimum lot area for lots in mixed-use and nonresidential zoning districts, including MM, within the TRO. The minimum lot width within the TRO is 35 feet. There is no maximum lot width for lots in mixed-use and nonresidential zoning districts within the TRO. All proposed lots have between 41 and 316 feet of frontage on public streets.

Intersection radii: Property lines corners are required to be rounded by arcs at street and alley intersections. The Plan Commission granted a waiver with primary plat approval to allow all lots to have right-angle corners to accommodate the goals of the development and in recognition that the street right-of-way designs provide adequate space for vehicle turning movements and sight lines.

Lot shape: All lots shall be designed with a depth-to-width ratio not to exceed four to one. All proposed lots comply with the required depth-to-width ratio. Some of the greenspace lots, such as lot 7, appear long and skinny. However, in the case of all of these lots the long dimension is the width along the adjacent street while the skinny dimension is the depth, meaning that their depth-to-width ratio is very small and well below the maximum of four to one.

Lot access: All new lots in the TRO shall have frontage on a public street right-of-way, per TRO section 20.02.050(b)(11)(B). All proposed lots have frontage on one of the existing or proposed public streets.

Stormwater Standards: All proposed subdivisions shall provide for the collection and management of all surface water drainage, and all subdivision requests shall include the submittal of a drainage plan to the City of Bloomington Utilities (CBU). The proposal indicates underground detention areas within greenspace lot 11. Although the proposed subdivision has not yet achieved CBU approval for the drainage plan, the plat provides enough area to meet the needs of required stormwater management facilities.

Right-of-Way Standards:

Street Layout: The proposed new streets are laid out in an orderly and logical manner, provide for pedestrian and vehicular safety, and provide direct access to existing public streets, as required by the UDO.

ROW width: West 2nd Street is designated as the General Urban street typology in the Transportation Plan, requiring an 84-foot right-of-way (42 feet from centerline). The adjacent segment of West 2nd Street is the subject of a street redesign project by the City of Bloomington Engineering Department, and plans for the West 2nd Street project show a right-of-way width requiring more than 42 feet from centerline near the intersection with South Rogers Street to accommodate turn lanes and other street infrastructure. The proposed plat dedicates additional right-of-way width along West 2nd Street to at least 42 feet from centerline per Transportation Plan guidance where the West Second Street project plans call for that much width or less, and dedicates greater width in accordance with the street project plans where those plan call for greater width. In this way, the proposed plat complies with both the Transportation Plan and the West 2nd Street project.

South Rogers Street is also designated as the General Urban street typology in the Transportation Plan, requiring an 84-foot right-of-way (42 feet from centerline). Condition 5 of the primary plat approval required 50 feet of right-of-way width from the centerline on the west side South Rogers Street between West 2nd Street and West University Street. The proposed plat dedicates additional right-of-way width along South Rogers Street to provide 50 feet from centerline north of West University Street and 42 feet ofrom centerline south of West University Street.

West 1st Street is designated as the Neighborhood Residential street typology with a 60-foot right-of-way in the Transportation Plan. The existing right-of-way of West 1st Street adjacent to the petition site is 66 feet, already more than called for in the Transportation Plan. No new right-of-way dedication is required. The proposal maintains the existing right-of-way of West 1st Street.

A new street grid for the former hospital site is identified in the Transportation Plan as part of new connection NC-45, with a Neighborhood Residential street typology and 60 feet of right-of-way width. The proposal dedicates 60 feet of new right-of-way width for South Fairview Street and South Jackson Street. As guided by the Master Plan, the proposed West University Street greenway follows a modified Shared Street typology with a 55-foot right-of-way width.

Street Trees: The minimum number of required street trees to be planted shall be one large canopy tree for every 30 feet of property that abuts a public right-of-way. The proposal shows 116 new street trees along both sides of the proposed new streets as well as along the west side of South Rogers Street, with typical spacing of 24 feet. The proposed trees are all permitted street tree species per the UDO, comprising ten species from eight different genera. In accordance with best practice and UDO requirements, no genus accounts for more than 20 percent of the street trees.

Alleys: Alleys must have a minimum 20-foot-wide right-of-way and a minimum 14-foot-wide pavement width. All alleys in the proposal show 20 feet of pavement width within 20-foot-wide rights-of-way.

Three of the proposed alleys are proposed to be dead-end alleys. Dead-end alleys are not prohibited by the UDO, but are discouraged where avoidable. One alley on the west side of the site connects with South Fairview Street but dead-ends at the west property line. The proposal shows that this alley is to be constructed as a stub alley providing the opportunity for an alley connection to the west in the future. A second alley in the northeast quadrant connects with South Fairview Street but appears to dead-end at lot 6. In fact, however, drive access will continue directly into the existing parking garage on lot 6 and the parking garage will serve as a possible turn-around for vehicles. A third alley in the southwest quadrant connects with South Jackson Street but dead-ends before it reaches Rogers Street due to the significant grade difference between the needed alley access to the Kohr building redevelopment and the level of Rogers Street below.

Street lighting: All subdivisions shall be required to have a street lighting plan approved by the City Engineering Department and submitted to the City Board of Public Works. Additionally, the street lighting plans must be accepted by the City Board of Public Works prior to secondary plat signing. In the TRO, street lighting must be pedestrian-scaled and no more than 15 feet in height. The proposal includes 49 lantern-style street lights mounted on 12-foot poles along South Rogers Street, South Jackson Street, and South Fairview Street. Along the West University Street greenway, the proposal includes 39 dual-fixture modern-style street lights mounted on 14-foot poles as well as lights strung on wires at 11 feet in height and ground lights. The street lighting plan has been submitted to the Board of Public Works for consideration of approval at the Board's meeting on June 18, 2024.

Environmental Considerations: A tree study of the larger Master Plan site was done, and no closed canopy areas were identified, though some specimen trees were located. The proposal maintains as many of the high-quality existing trees as possible. There are no other known sensitive environmental features.

Utilities: The proposal shows public water and sanitary sewer service to all lots. A utility plan must be approved by City of Bloomington Utilities (CBU) prior to secondary plat approval. The petitioner has submitted the proposed plans to CBU and is working toward approval. After secondary plat approval and recording, during development of lots in the subdivision, CBU approval will be required before any permits for development are issued for the lots in the subdivision.

CONCLUSION: The secondary plat for the proposed subdivision complies with all standards in UDO and meets the design and conditions of the associated primary plat. As part of the redevelopment of the area included in the Bloomington Hospital Site Redevelopment Master Plan, the subdivision will set up Hopewell West with improved public ways and new amenities, as well as create new developable parcels.

RECOMMENDATION: The Planning and Transportation Department recommends that the Plat Committee approve the secondary plat of DP-23-24 / PLAT2024-05-0029.

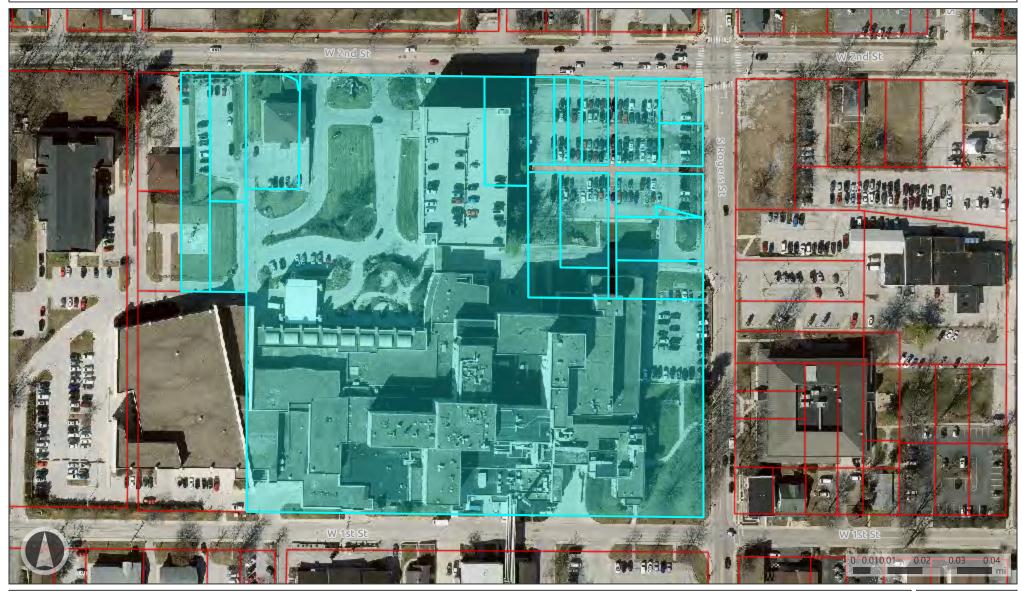


Hopewell West Location Map





Hopewell West Context Aerial



Map Legend

Parcels



April 19, 2024

Gabriel Holbrow, AICP Zoning Planner Planning & Transportation Department City of Bloomington, Indiana

Re: Hopewell West Petitioner's Statement - Secondary Plat

Mr. Holbrow,

Please accept this letter as the Petitioner's Statement for the Hopewell West Subdivision Secondary Plat submission. CrossRoad Engineers, P.C. and the City of Bloomington Engineering Department presents this statement on behalf of the City of Bloomington Redevelopment Commission. The goal of this project is to subdivide the former Bloomington Hospital site into new streets, alleys, and lots that will facilitate redevelopment in accordance with the vision established by the community in the Bloomington Hospital Site Redevelopment Master Plan. The Kohr building and the existing parking garage on the site will be retained. Utility infrastructure and stormwater control will also be part of this project.

Primary Plat Approval was achieved on July 11, 2023. We have updated the Secondary Plat according to all conditions outlined in the Notice of Approval. As recommended by the Hopewell Development Advisory Group, we have combined lots 3-6, 9-13, and 18-26 (lot numbers as shown on the approved Primary Plat) into 3 larger lots. This provides one single lot having a depth-to-width ration not exceeding four-to-one, meeting the spirit of condition 4. Further, we wanted to note three waivers were granted. The first waiver was to allow the subdivision of a parent tract greater than three acres. The second was a waiver to allow the greenspace lots to lack alley access. The third was a waiver to allow right-angle corners of development lots at street and alley corners. The fourth and fifth waivers were alterations to tree plot and pedestrian facility widths in the right-of-way.

We thank you for your thoughtful consideration of this Secondary Plat Submission. If you have any questions or need additional information, please feel free to contact me at your convenience.

Sincerely,

Androw J. Wolf, PE

CrossRoad Engineers, P.C.

317-780-1555 x124

awolf@crossroadengineers.com

Kendall Knoke, PE

City of Bloomington Engineering Dept.

812-339-3467

kendall.knoke@bloomington.in.gov

24-36 RESOLUTION OF THE REDEVELOPMENT COMMISSION OF THE CITY OF BLOOMINGTON INDIANA

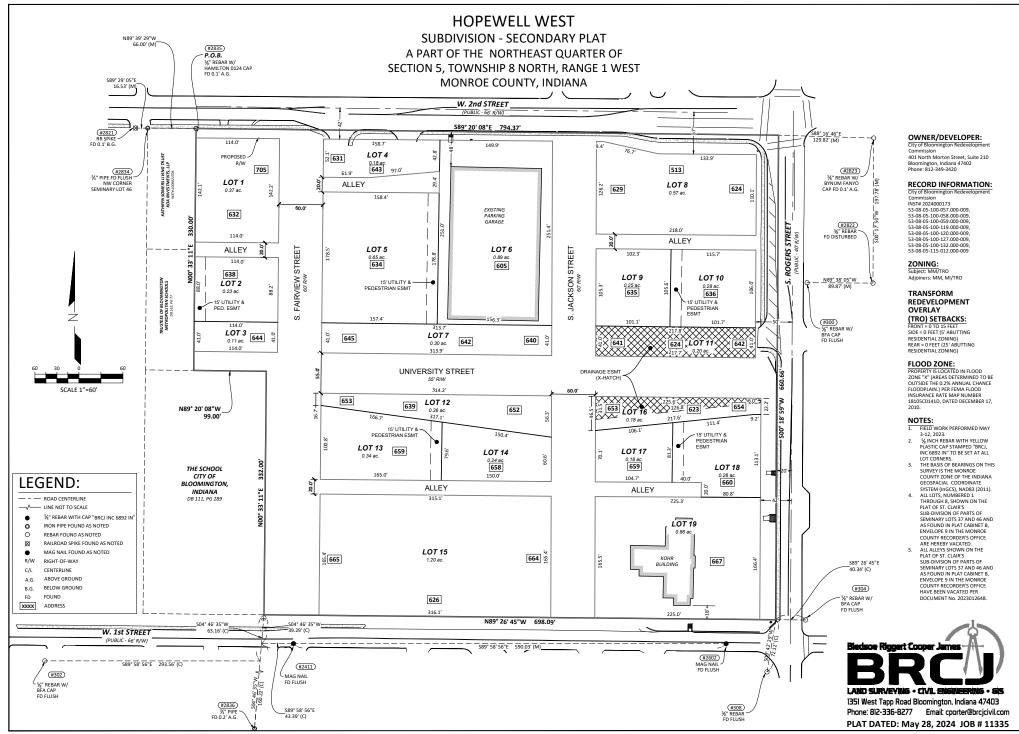
APPROVAL OF SECONDARY PLAT FOR HOPEWELL WEST

- WHEREAS, pursuant to Indiana Code 36-7-14 *et seq.*, the Redevelopment Commission of the City of Bloomington ("RDC") and the Common Council of the City of Bloomington created an economic development area known as the Consolidated Economic Development Area ("Consolidated TIF"); and
- WHEREAS, in <u>Resolution 18-10</u>, the RDC approved a Project Review and Approval Form ("Form") which sought the support of the RDC for the purchase and redevelopment the Old Bloomington Hospital Site ("Hopewell"); and
- WHEREAS, part of the redevelopment of the site includes making infrastructure improvements to the Hopewell western parcels ("Hopewell West"), which included the former main building that was IU Health Bloomington Hospital ("Project"); and
- WHEREAS, in Resolution 23-48, the RDC approved a primary plat for Hopewell West; and
- WHEREAS, City staff have prepared a secondary plat for Hopewell West, which is attached to this Resolution as Exhibit A; and
- WHEREAS, if approved, the secondary plat will be submitted for approval to the Plat Committee of the Bloomington Plan Commission or other designee by the Plan Commission to complete the plat approval process.

NOW, THEREFORE, BE IT RESOLVED BY THE BLOOMINGTON REDEVELOPMENT COMMISSION THAT:

- 1. The RDC reaffirms its support of the Project and reiterates that it serves the public's best interests.
- 2. The RDC approves the secondary plat for Hopewell West and authorizes its submission for all necessary approvals.
- 3. The RDC authorizes the RDC President, or any available and duly elected RDC officer, to sign all documents necessary to record the secondary plat for Hopewell West.

BLOOM	INGTON REDEVELOPMENT COMMISSIO
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Deborah	Hutton, President
ATTES	Γ:
Sue Sga	mbelluri, Secretary
	4-22-24
Date	



2024

HOPEWELL WEST

SUBDIVISION - SECONDARY PLAT A PART OF THE NORTHEAST QUARTER OF SECTION 5, TOWNSHIP 8 NORTH, RANGE 1 WEST MONROE COUNTY, INDIANA

LEGAL DESCRIPTION

A part of Seminary Lots 37 and 46 and St. Clair's Subdivision of parts of Seminary Lots 37 and 46 in the City of Bloomington, Monroe County, Indiana, and more particularly described by Christopher L. Porter, LS21200022, on May 15, 2023, as part of Bledsoe Riggert Cooper James, Inc. Job Number 11335, as follows:

Commencing at a 3/4-inch diameter iron pipe marking the northwest corner of Seminary Lot 46; thence along the north line of said Lot 46 SOUTH 80 degrees 39 minutes 29 seconds EAST a distance of 66.00 feet to a 5/8-inch diameter rebar with cap and the Point of Beginning; thence continuing along said north line SOUTH 89 degrees 20 minutes 08 seconds EAST a distance of 794.37 feet to the northeast corner of Lot 1 of St. Clair's Subdivision as recorded in Plat Book 15, Page 65 in the Monroe County Recorder's office and the west right of way line of Rogers Street; thence along said right of way line of Rogers Street; thence along said right of way line of First Street: thence along said right of way line of First Street: thence along said right of way line NORTH 89 degrees 26 minutes 45 seconds WEST a distance of 69.06 feet to the north right of way line of First Street: thence along said right of way line NORTH 89 degrees 26 minutes 45 seconds WEST a distance of 332.00 feet; thence NORTH 89 degrees 20 minutes 80 seconds WEST a distance of 99.00 feet) to the cast line of long the cast line of said Deed Book 112, Page 77; thence along the east line of said Deed Book and the east line of Instrument Number 2004021706 NORTH 00 degrees 33 minutes 11 seconds EAST a distance of 330.00 feet to the point of beginning, containing 11.33 crees, more or less, more or le

This description includes the platted alleys in St. Clair's Subdivision.

REPORT OF SURVEY

In accordance with Title 865, 1-12-1 through 1-12-30 of the Indiana Administrative Code, the following observations and opinions are submitted regarding the various uncertainties in the locations of the lines and corners established on this survey as a result of:

- (a) Reference monuments of record
- b) Title documents of record
- (c) Evidence of active lines of occupation
- (d) Relative Positional Accuracy "RPA"

The Relative Positional Accuracy "RPA" (due to random errors in measurement) of this survey is within that allowable for an Urban survey (0.07 feet (21 millimeters) plus 50 parts per million) as defined in IAC, Title 865 ("Relative Positional Accuracy" means the value expressed in feet or meters that represents the uncertainty due to random errors in measurements in the location of any point on a survey relative to any other point on the same survey at the 95 percent confidence level.).

In regard to "ACTIVE LINES OF OCCUPATION", point (c) above: ACTIVE refers to lines which are marked by visible, obvious, well defined and maintained, man-made or placed objects, such as, but not limited to, fences, hedges and retaining walls. The uncertainty cited for a line of occupation is general in nature and is NOT intended to be specific for every point along the line. Therefore, portions of the occupation line may vary from the surveyed line by a distance greater or less than uncertainty cited in this report.

This is a partial Retracement Survey and an Original Survey performed at the request of the City of Bloomington Redevelopment Commission.

The surveyed property was in the name of Bloomington Hospital, Inc. (Deed Book 364, Page 240, Instrument No. 2004018581 and Instrument Number 2010019969) at the time the field work was conducted, and when the legal description and report of survey were written. At the time of certification of this plat, the property is in the name of The City of Bloomington, Indiana, by and through the Bloomington Redevelopment Commission (Instrument No. 2024000173).

The field work was performed May, 2023.

SURVEYS & PLATS OF RECORD:

- ALTA/NSPS Land Title Survey for Indiana University Health, Inc. by Terry D. Wright, Hamilton Designs Job Number 2018-147, dated May 25, 2018, provided by Indiana University Health, Inc.
- 2. Plat of Seminary Square and Lots, found in Plat Cabinet B, Envelope 5 in the Monroe County Recorder's office.
- Plat of St. Claire's Subdivision of Parts of Seminary Lots 37 and 46, found in Plat Cabinet B. Envelope 9 in said Recorder's office.
- 4. Survey of Seminary Lots 11, 12, 13, 14 and Part of Lot 10 and Part of Seminary Lot 37 by Charles D. Graham, found recorded as Instrument Number 2021024040 in said Recorder's Office.

MONUMENTS FOUND:

- 300. A 5/8-inch diameter rebar with Bynum Fanyo Associates cap was found flush with grade. This monument is shown as number 500 on the Graham survey.
- 302. A 5/8-inch diameter rebar with Bynum Fanyo Associates cap was found flush with grade. This monument is shown on the Hamilton survey as the northwest corner of Tract 6, PCL 3 per Deed Record 371. Page 479.
- 304. A 5/8-inch diameter rebar with Bynum Fanyo Associates cap was found flush with grade. This monument is shown as number 504 on the Graham survey.
- 308. A 5/8-inch diameter rebar with illegible cap was found flush with grade. The origin of this monument is unknown.
- 2821. A railroad spike was found 0.1 foot below grade. The origin of this monument is unknown.

2822. A 5/8-inch diameter rebar was found disturbed. This monument is shown as number 501 on the Graham survey.

2823. A 5/8-inch diameter rebar with Bynum Fanyo Associates cap was found 0.1 foot above grade. This monument is shown as number 502 on the Graham survey.

2834. A 3/4-inch diameter iron pipe was found flush with grade and accepted as the northwest corner of Seminary Lot 46 per survey 1.

2835. A 5/8-inch diameter rebar with Hamilton 0124 cap was found 0.1 foot above grade and accepted as the northwest corner of Instrument Number 2004018581 per survey 1.

2836. A 3/4-inch diameter iron pipe was found 0.2 feet above grade. This monument is shown on the Hamilton survey as the southeast corner of Tract 6, PCL 2 per Deed Record 371, Page 478.

DEED ANALYSIS:

No discrepancies were found when comparing the legal descriptions for the western adjoiners with the Bloomington Hospital, Inc. descriptions.

ESTABLISHMENT OF LINES AND CORNERS:

Monument 2835 as held for the geometry shown on the Hamilton survey. Said geometry was then rotated to monument 2836 to establish the perimeter lines of the Bloomington Hospital, Inc. parcels.

As a result of the above observations, it is my opinion that the uncertainties in the location of the lines and corners established on this survey are as follows:

Due to Availability and condition of reference monuments: Up to 1.5 feet when comparing the distance between monuments 2835 and 2836 calculated per the Hamilton survey with the measured distance.

Due to Occupation or possession lines: No discrepancies noted.

Due to Clarity or ambiguity of the record description used and of adjoiners' descriptions and the relationship of the lines of the subject tract with adjoiners' lines: No discrepancies noted.

EASEMENT DEFINITIONS

Drainage Easements: (A) Shall be required for any surface swales or other minor drainage improvements that are intended to serve the lots on which they are located. (B) Shall prohibit any alteration within the easement that would hinder or redirect flow. (C) Shall provide that the owner of the lot on which the easement is placed shall be responsible for maintenance of the drainage features within such easement. (D) Shall be enforceable by the City utilities department and by owners of properties that are adversely affected by conditions within the easement. (E) Shall allow the City utilities department to enter upon the easement for the purpose of maintenance, to charge the costs of such maintenance to the responsible parties, to construct drainage facilities within the easement, and to assume responsibility for the drainage features at its discretion.

Utility Easements: (A) Shall allow both private and public utility providers access associated with the installation, maintenance, repair, or removal of utility facilities. (B) Prohibits the placement of any unauthorized obstruction within the easement area unless authorized by the City utilities department and the easement holder(s).

Pedestrian Easements: (A) Grants the general public the right to access the pedestrian easement for purposes of walking, running, bicycling, skating, or using small motorized and non-motorized vehicles approved by the City, (B) Grants the City the right to construct, alter, repair, maintain, or remove improvements within the easement area. (C) Prohibits the placement of any obstruction within the nedestrian easement.

OWNER CERTIFICATION

City of Bloomington Redevelopment Commission, Owner of the real estate shown and described herein, does hereby certify, layoff, and plat (19) tracts, numbered 1-19.

Rights-of-way not heretofore dedicated are hereby dedicated to the public. In accordance with this plat and certificate, this plat shall be known as Hopewell West Subdivision.

IN WITNESS WHEREOF, the undersigned Owner set their hand and seal this	day of
2024	

Deborah Hutton, President

City of Bloomington Redevelopment Commission

WITNESS my hand and Notarial Soal this day of

STATE OF INDIANA COUNTY OF MONROE

Before me, a Notary Public in and for said County and State, personally appeared City of Bloomington Redevelopment Commission, owner, who acknowledged the execution of the above referenced plat, to be their voluntary act for the uses and purposes therein set forth.

WITNESS ITTY Harid and Notatian Sear tillsday or	,2024	
Notary Public (Signature)		
Notary Public (Printed Name)		
My Commission Expires:		
My County of Residence:		

PLAN COMMISSION AND BOARD OF PUBLIC WORKS

Under the authority provided by Chapter 174, Acts of 1947, enacted by the General Assembly of the State of Indiana and ordinance adopted by the Common Council of the City of Bloomington, Indiana, this plat was given approval by the City of Bloomington as follows:

APPROVED BY THE PLAT COMMITTEE AT A MEETING HELD:	, 2024

SURVEYOR'S CERTIFICATION

Director of Planning & Transportation Department

This survey was executed according to survey requirements contained in Section 1 through 19 of 865 IAC 1-12.

This certification does not take into consideration additional facts that an accurate and correct title search and/or examination might disclose.

Evidence of easements have not been located in the field and are not shown on this survey drawing.

Subject to the above reservation, I hereby certify that the survey work performed on the project shown hereon was performed either by me or under my direct supervision and control and that all information shown is true and correct to the best of my knowledge and belief.

Certified this 28th day of May, 2024.

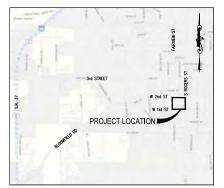
Christopher L. Porter.

Christopher L. Porter Professional Surveyor No. LS21200022 State of Indiana

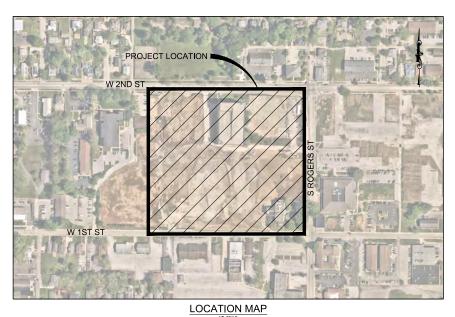




60% CONSTRUCTION PLANS HOPEWELL INFRASTRUCTURE & SITE WEST BLOOMINGTON, INDIANA



VICINITY MAP



SHEET#	SUBJECT
100	TITLE SHEET & INDEX
200	TYPICAL SECTIONS
300	TOPOGRAPHIC SURVEY
400-405	SITE PLANS
500	GRADING PLAN
501	INTERSECTION GRADING PLAN
600-605	ROAD PLAN & PROFILE
700	DETENTION BASIN PLAN
800-804	MISCELLANEOUS DETAILS
E101-E103	ELECTRICAL PLANS
LA101-LA102	LANDSCAPE LAYOUT
LA201-LA202	LANDSCAPE GRADING
LA301-LA304	LANDSCAPE PLANS
XS1-XS10	S FAIRVIEW ST. CROSS SECTIONS
XS11-XS24	S JACKSON ST. CROSS SECTIONS
XS25-XS34	W UNIVERSITY ST. CROSS SECTIONS
XS35-XS44	S ROGERS ST. CROSS SECTIONS
XS45-XS49	"F&J ALLEY" CROSS SECTIONS
XS50-XS53	"F EAST ALLEY" CROSS SECTIONS
XS54-XS57	"J EAST ALLEY" CROSS SECTIONS

UTILITY CONTACTS					
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UTILITY	COMPANY	CONTACT	PHONE	EVAL	
COMMUNICATIONS	AT&T INDIANA BELL	ANDY MULLIS	(812)-334-4597	am5495@att.com	
COMMUNICATIONS	COMCAST	SCOTT TEMPLETON	(812)-332-4152	scott_templeton@comcost.com	
COMMUNICATIONS	ITS	DIRECTOR	(812)-349-3454	-	
COMMUNICATIONS	TCI OF INDIANA	-	(812)-332-9185	-	
ELECTRIC	DUKE EMERGY	BRANDON WLSON	(812)-336-6371	brandon.wison2@duke-energy.com	
SANITARY & WATER	UTILITIES DEPARTMENT	UTILITY ENGINEER	(812)-339-1444	-	
GAS	CENTERPORT GAS	SUPERINTENDENT	(812) 330-4008	publicaroject@centerpointenergy.com	
FIRE & AMBULANCE	ELOOMINGTON FIRE DEPARTMENT	FIRE CHEF	(812)-332-9763		
POLICE	ELOOMINGTON POLICE DEPT.	CHIEF DIEXOFF	(812)-349-4477	-	
UTILITIES	BOARD OF PUBLIC WORKS	ADAM WASON	(812)-349-3410	-	

INDIANA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS DATED 2024 TO BE USED WITH THESE PLANS.

CONSTRUCTION SPECIFICATIONS FOR CITY OF BLOOMINGTON UTILITIES (LATEST EDITION) SHALL BE USED FOR ALL WASTEWATER, WATER AND STORM INFOASTBUCTURE

OWNER/DEVELOPER

CITY OF BLOOMINGTON
401 N. MORTON ST.
BLOOMINGTON, IN 47404
PHONE: (812) 349-3913
CONTACT: KENDALL KNOKE
EMAIL: kendall.knoke@bloomington.in.gov

ENGINEER

CROSSROAD ENGINEERS, PC
115 N. 17TH AVENUE
BEECH GROVE, IN 46107
PHONE: (317) 780-1555
CONTACT: ANDREW J. WOLF
EMAIL: awolf@crossroadengineers.com

LANDSACAPE ARCHITECT

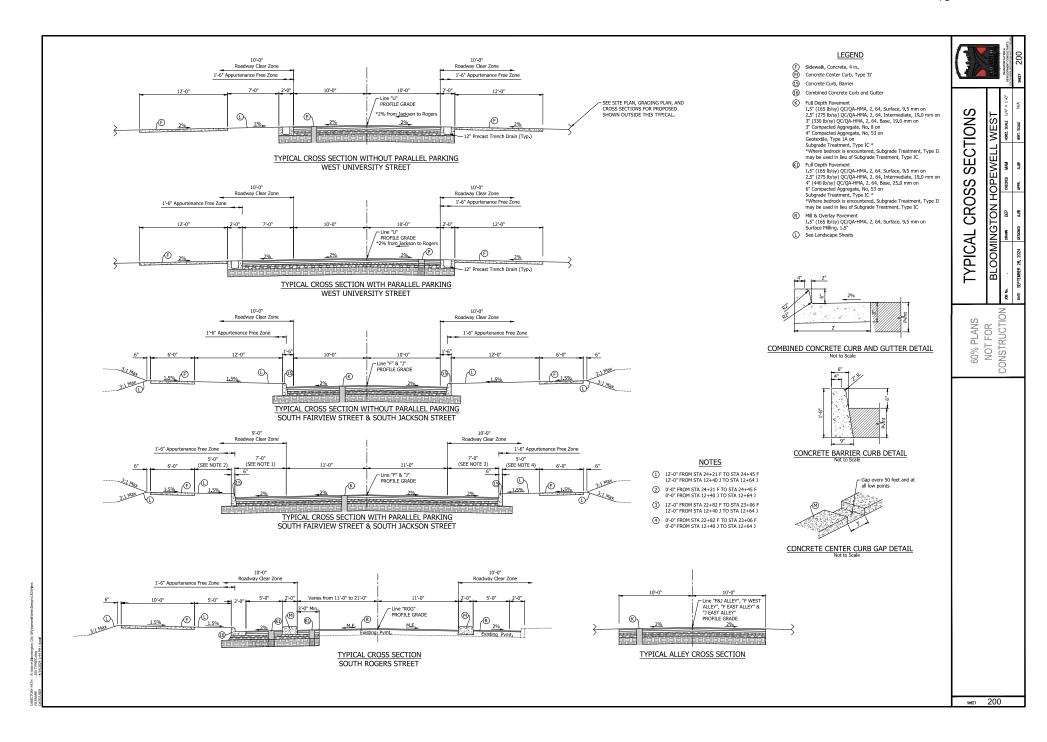
LEGAL DESCRIPTION

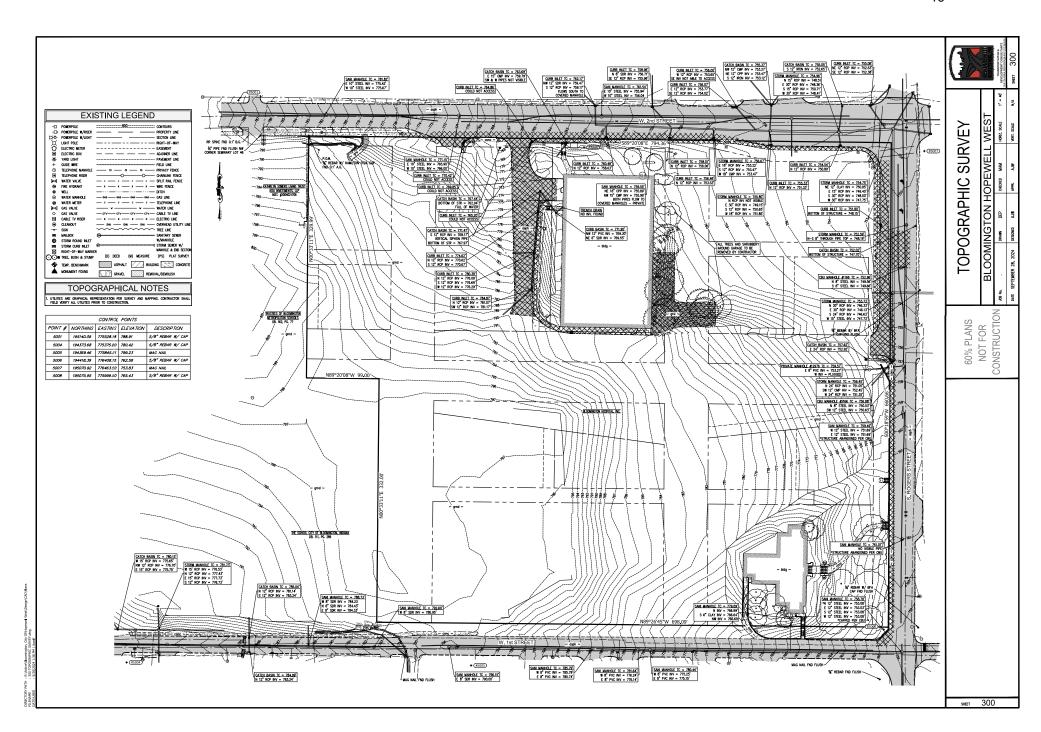
RUNDELL ERNSTBERGER ASSOCIATES
618 EAST MARKET STREET
INDIANAPOLIS, IN 46202
PHONE: (317) 263-0127
CONTACT: CECIL PENLAND, PLA, ASLA
EMAIL: cpenland@reasite.com

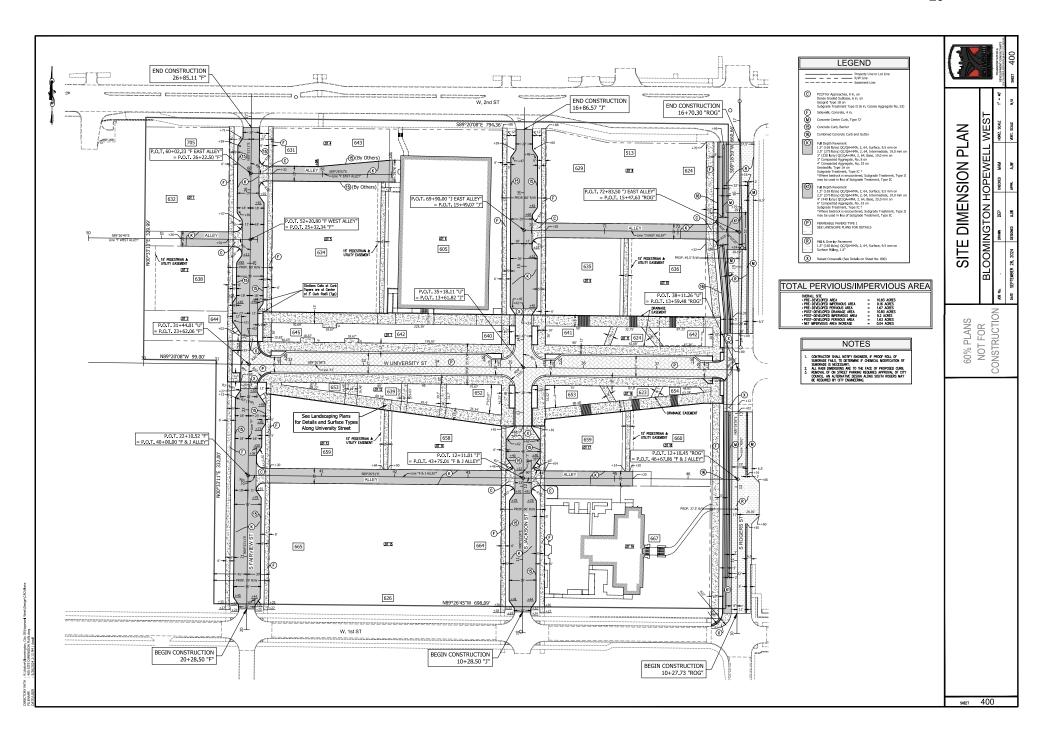
TRAFFIC DATA	S FAIRVIEW ST	S JACKSON ST	W UNIVERSITY ST	ALLEYS	S ROGERS ST
A.A.D.T. (2022)	- V.P.D.	- V.P.D.	- V.P.D.	- V.P.D.	8,553 V.P.D.
A.A.D.T. (2042)	- V.P.D.	- V.P.D.	- V.P.D.	- V.P.D.	- V.P.D.
D.H.V (2042)	- V.P.H.	- V.P.H.	- V.P.H.	- V.P.H.	- V.P.H.
DIRECTIONAL DISTRIBUTION	- POS	- POS.	- POS	- POS.	- POS.
TRUCKS	- A.A.D.T.	- A.A.D.T.	- A.A.D.T.	- A.A.D.T.	- A.A.D.T.
	- D.H.V.	- D.H.V.	- D.H.V.	- D.H.V.	- D.H.V.
DESIGN DATA					
DESIGN SPEED	20 M.P.H.	20 M.P.H.	20 M.P.H.	20 M.P.H.	25 M.P.H.
PROJECT DESIGN CRITERIA	NEW CONSTRUCTION	NEW CONSTRUCTION	NEW CONSTRUCTION	NEW CONSTRUCTION	3R
FUNCTIONAL CLASSIFICATION	LOCAL (NEIGHBORHOOD RES.)	LOCAL (NEIGHBORHOOD RES.)	LOCAL (SHARED STREET)	LOCAL (ALLEY)	SECONDARY ARTERIAL (GENERAL URBAN STREET)
RURAL/URBAN	URBAN (BUILT-UP)	URBAN (BUILT-UP)	URBAN (BUILT-UP)	URBAN (BUILT-UP)	URBAN (BUILT-UP)
TERRAIN	ROLLING	ROLLING	ROLLING	ROLLING	ROLLING
ACCESS CONTROL	NONE	NONE	NONE	NONE	NONE

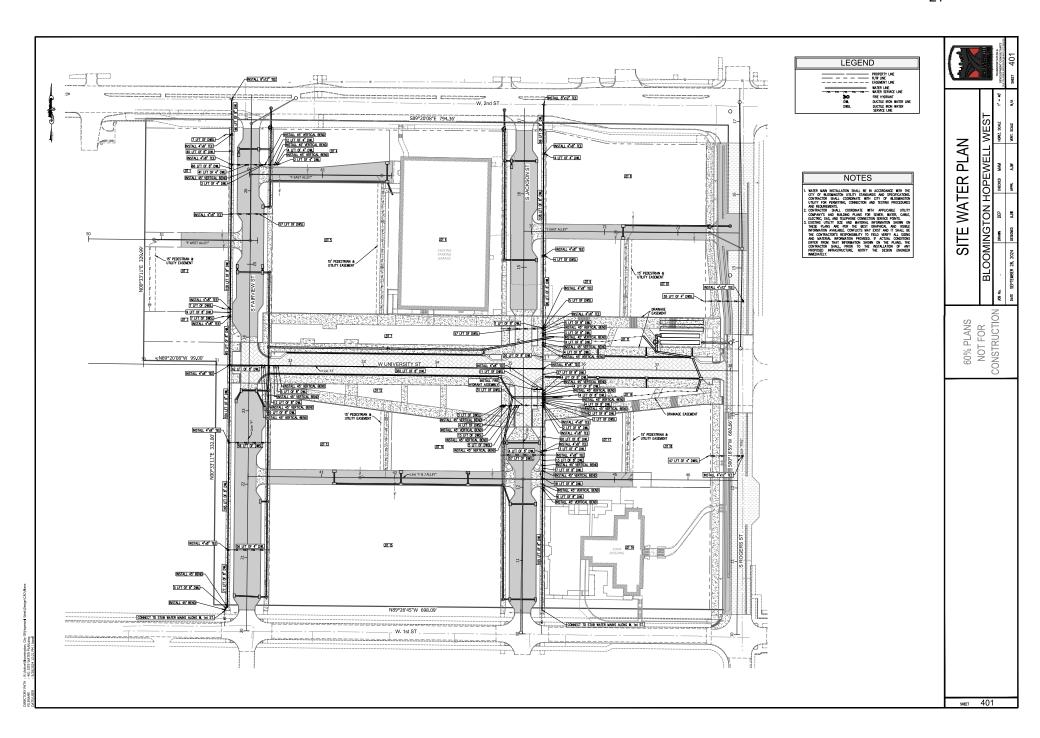
FLOODPLAIN INFORMATION

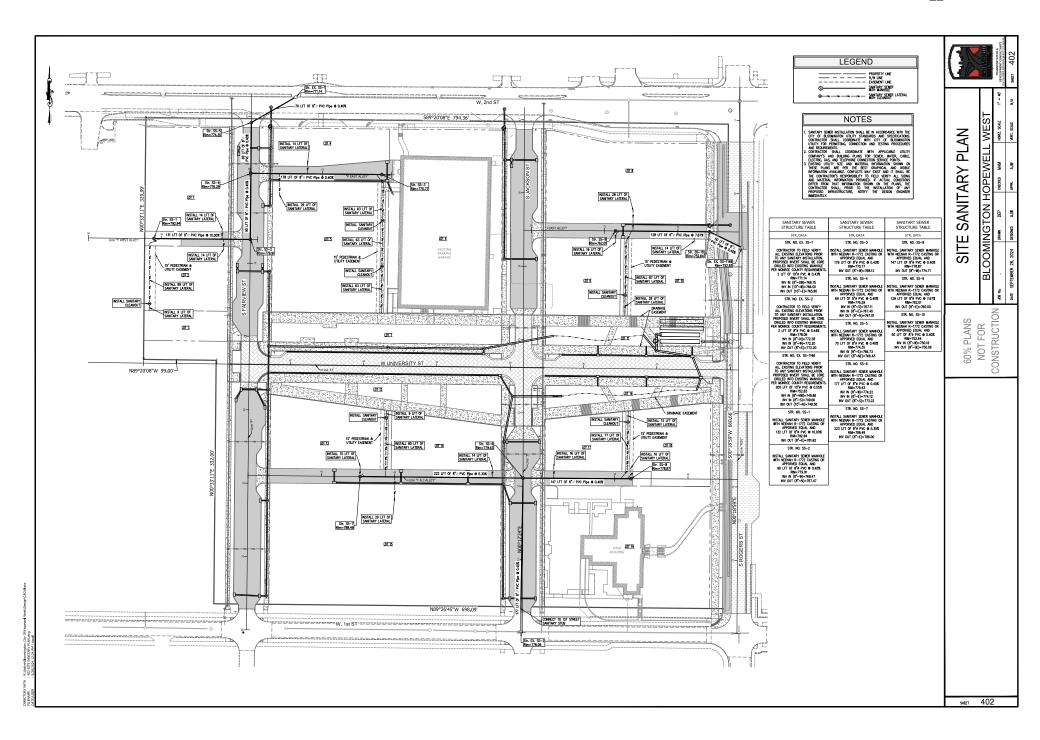
60% PLANS
NOT FOR

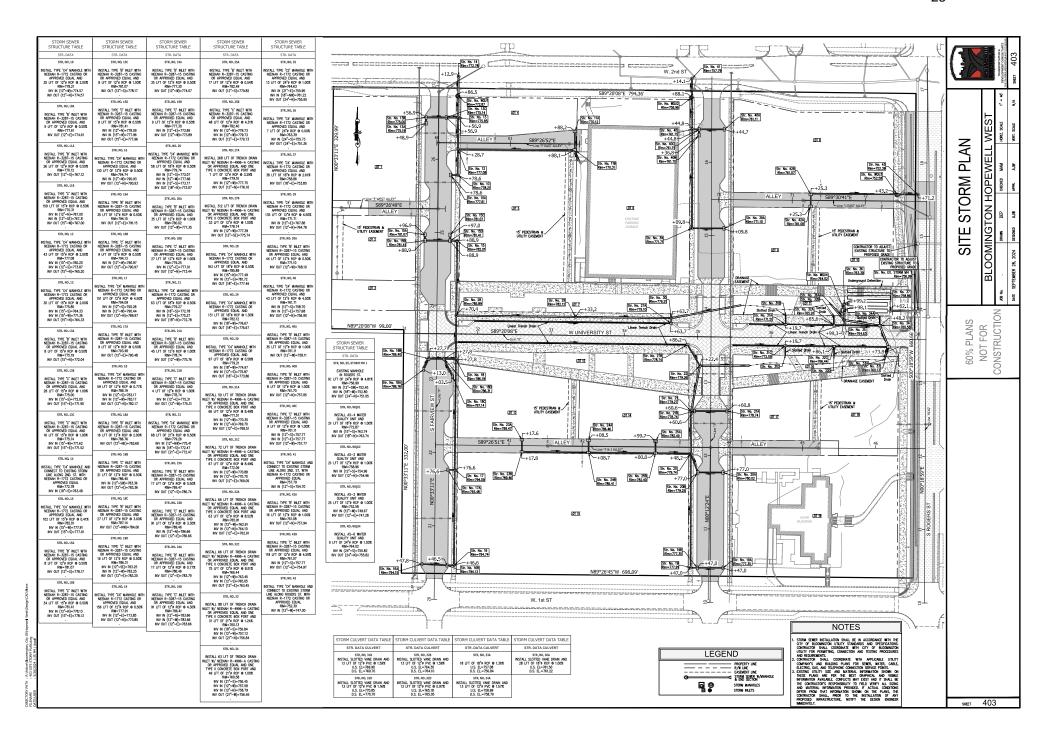


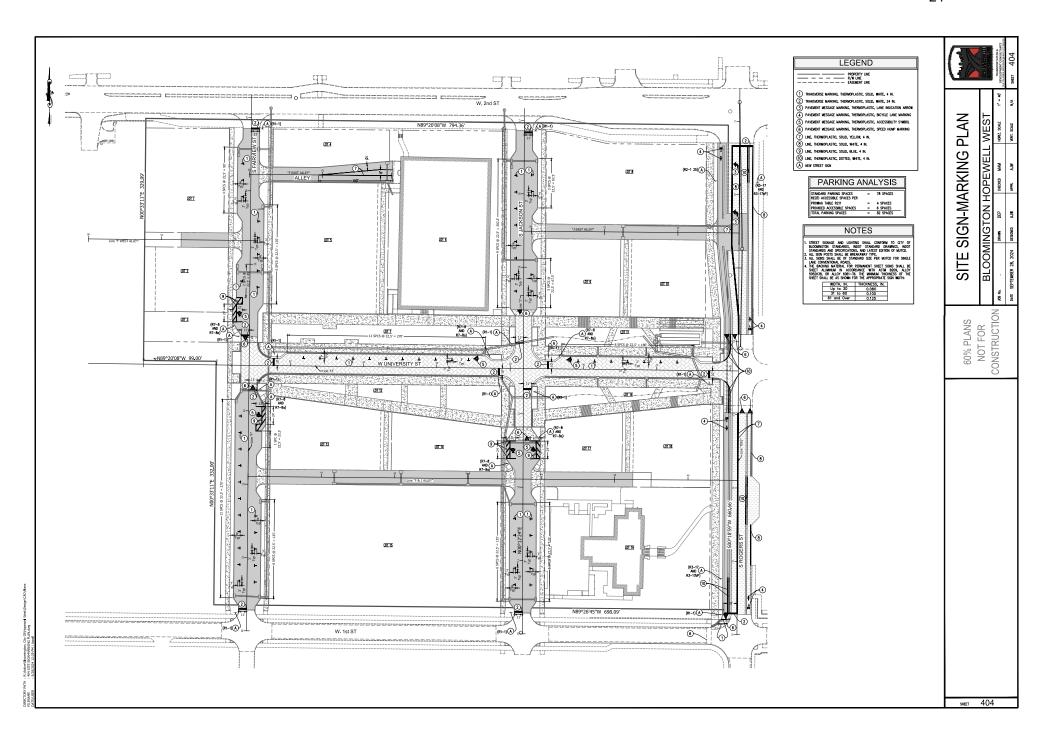


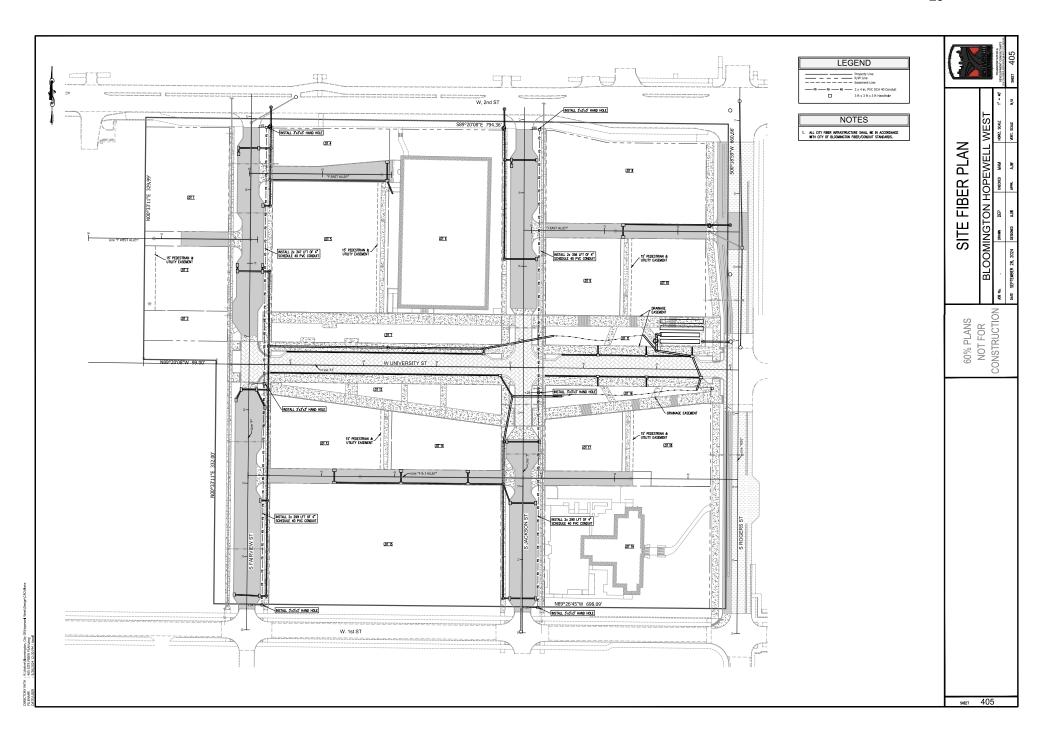


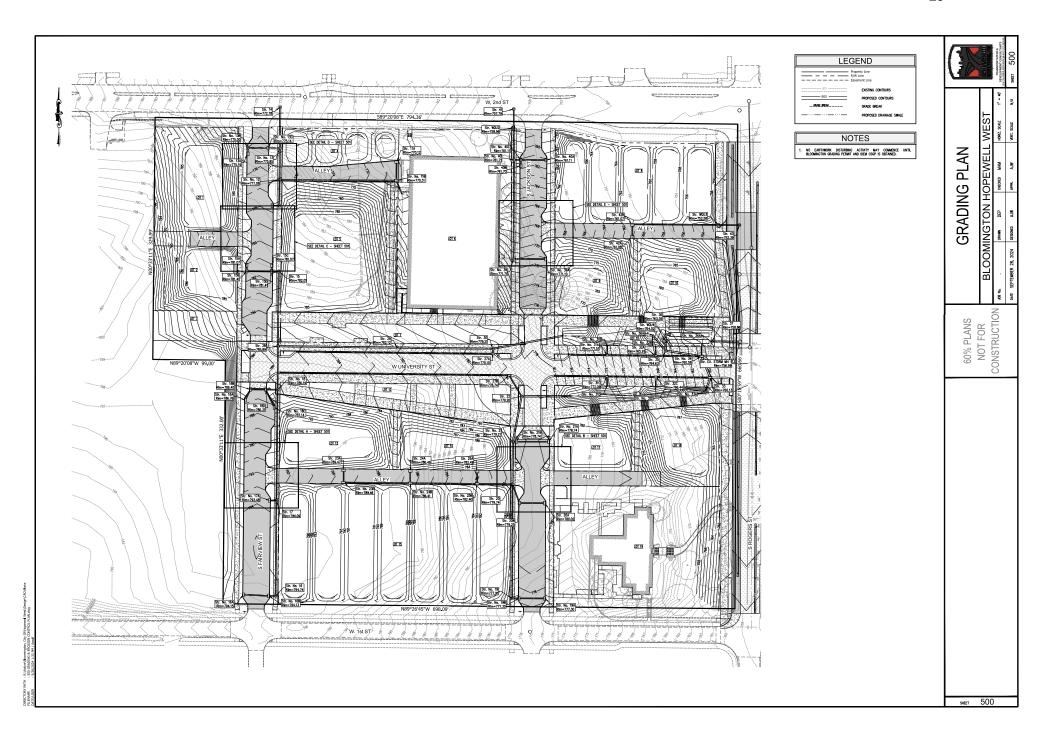




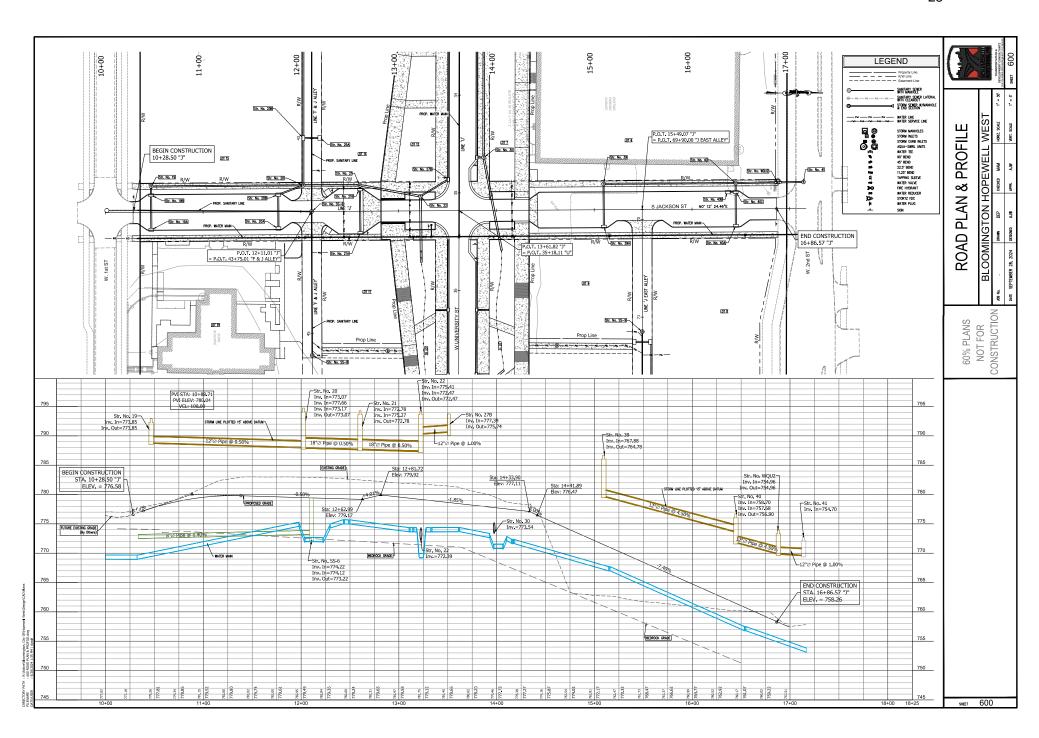


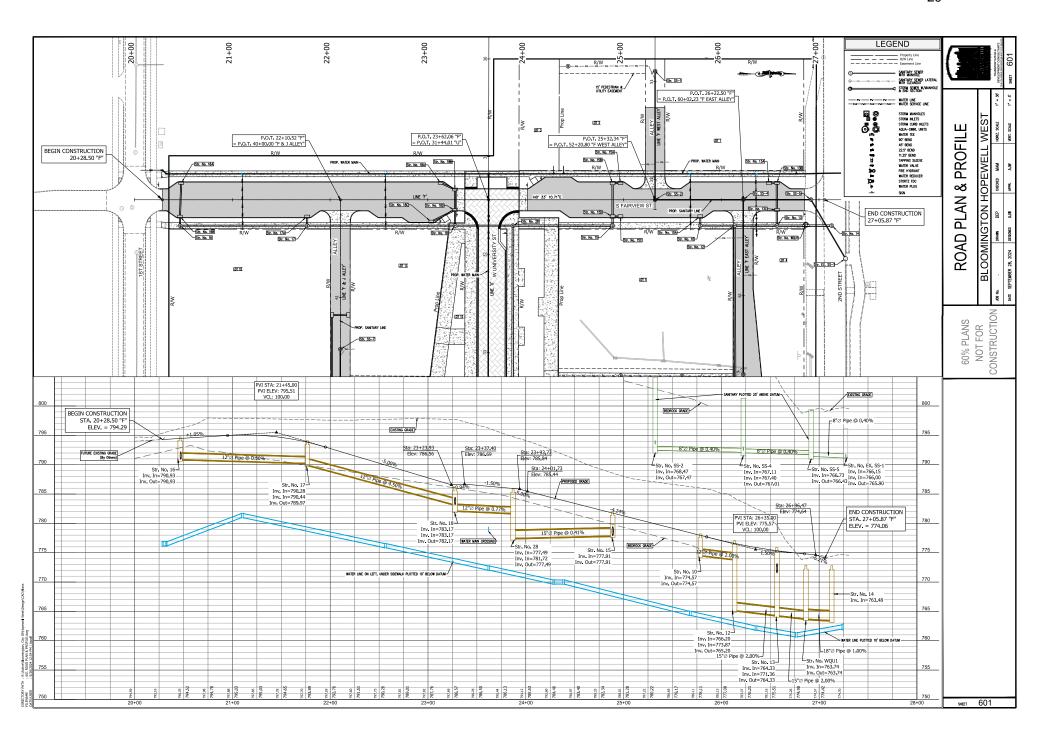


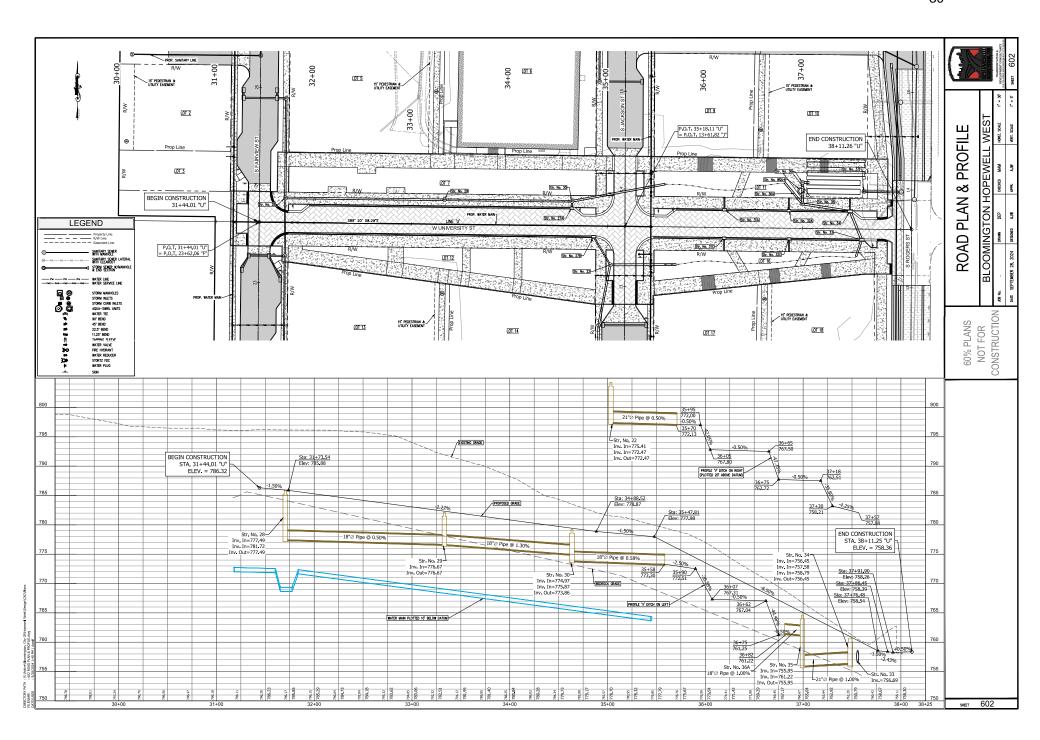


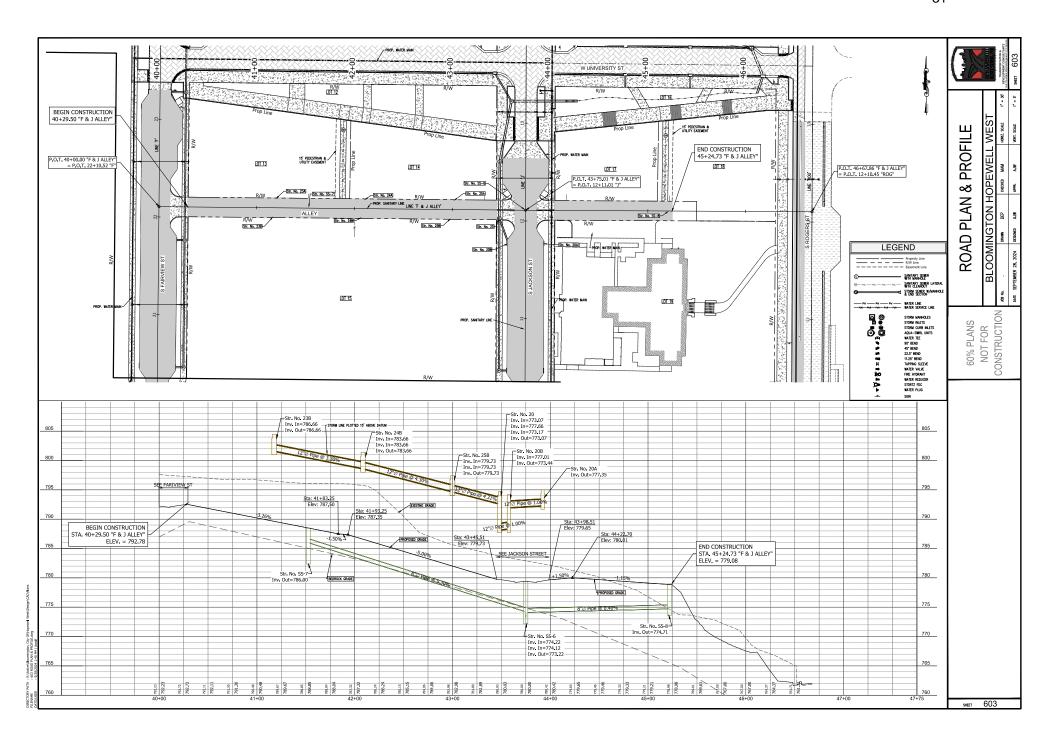


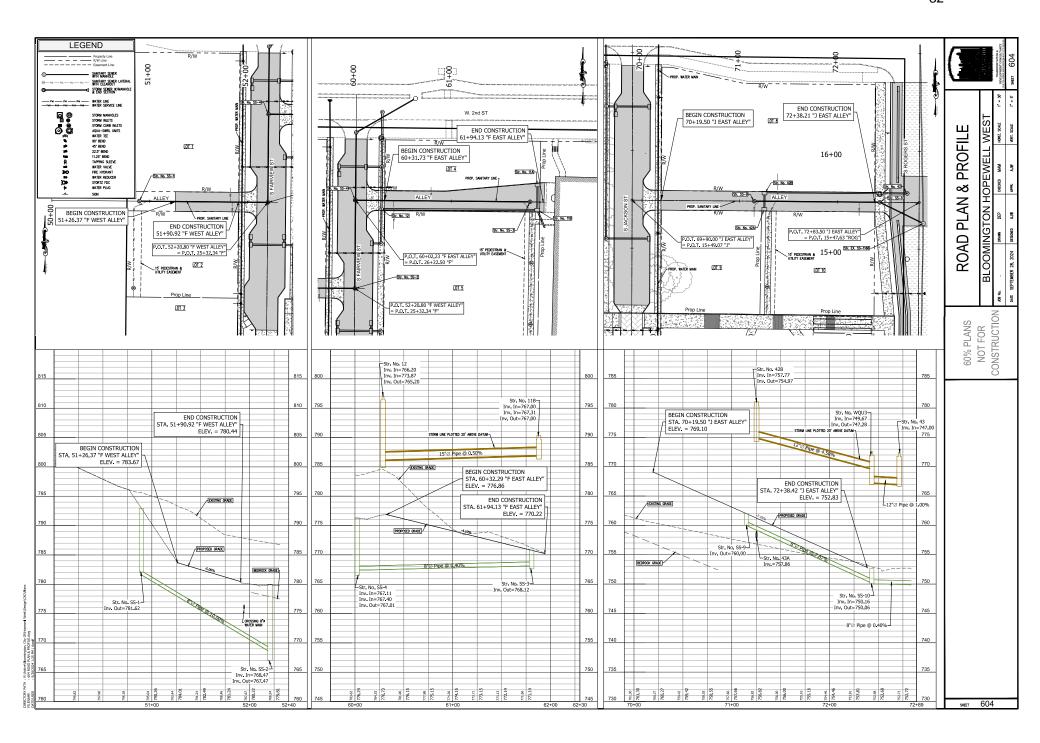


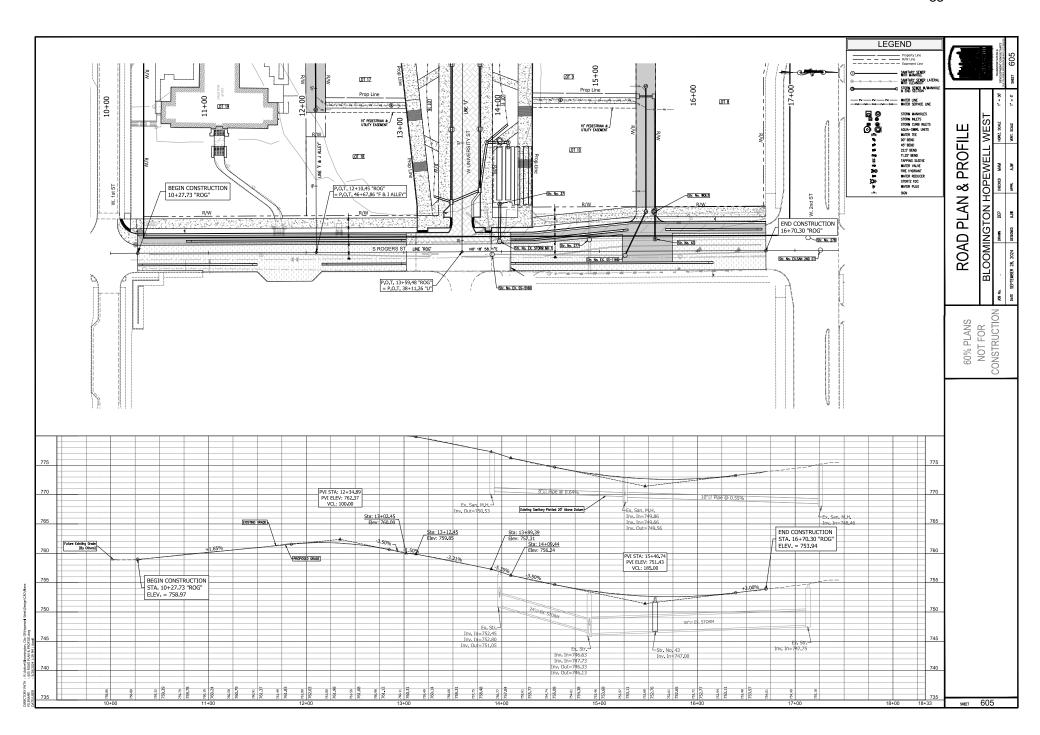


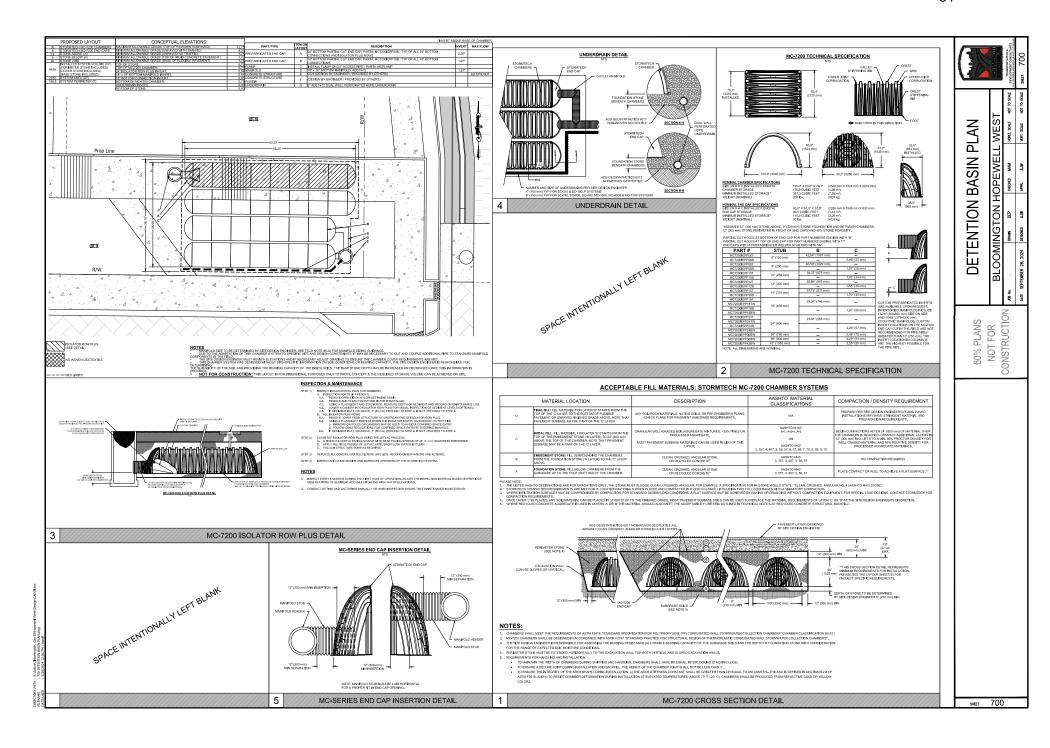


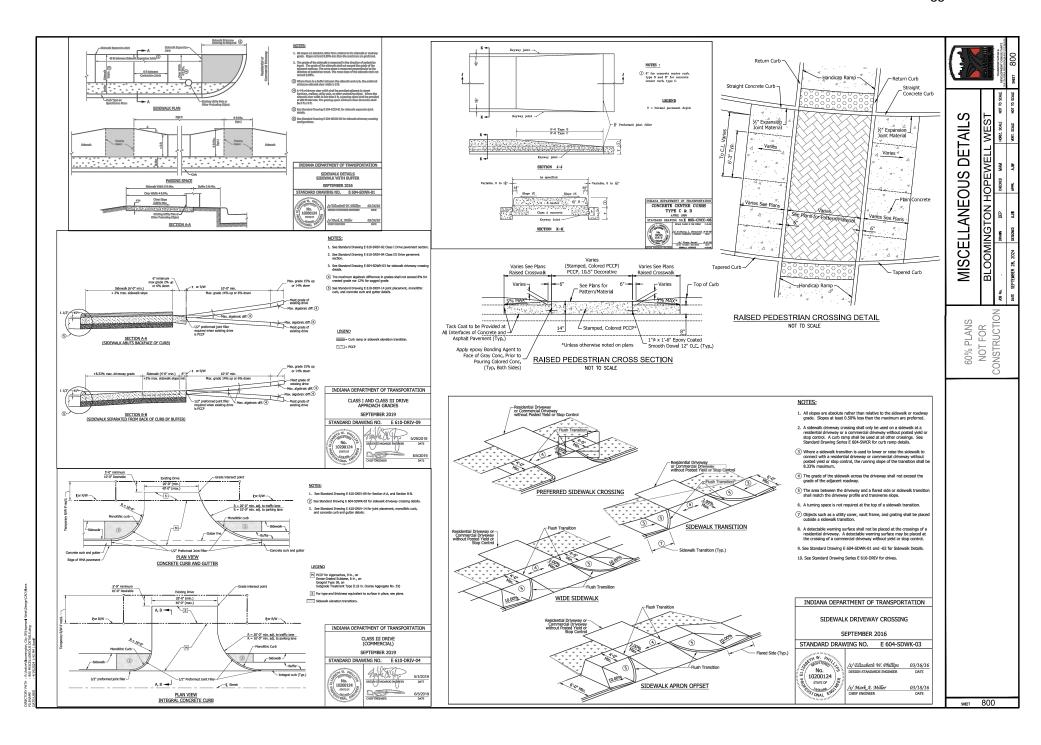


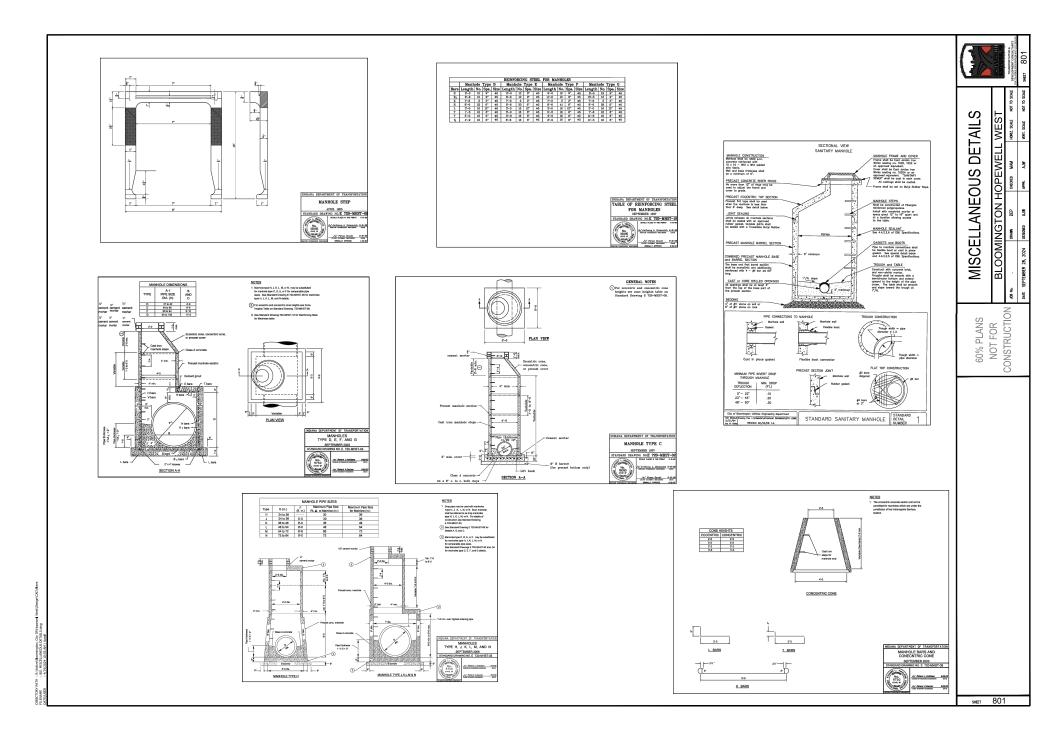


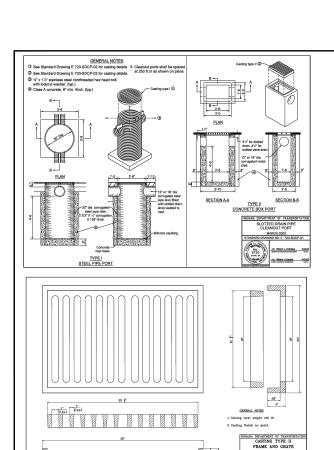


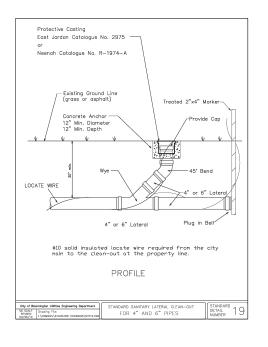


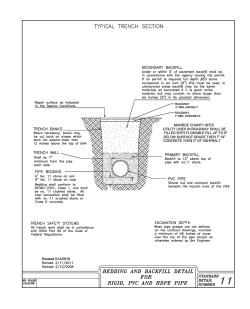


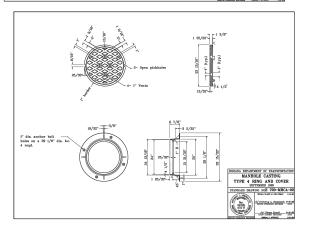






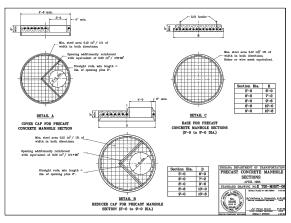


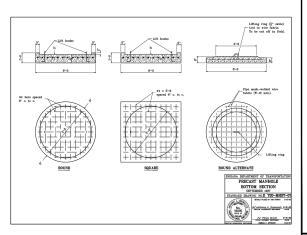




CASTING TYPE II

(100 mm)







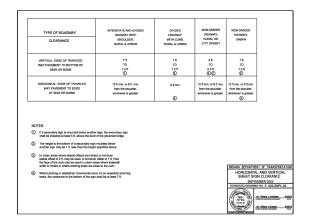
MISCELLANEOUS DETAILS

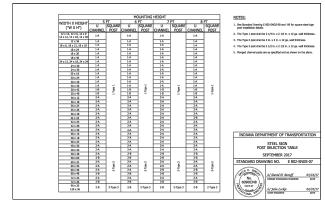
BLOOMINGTON HOPEWELL WEST

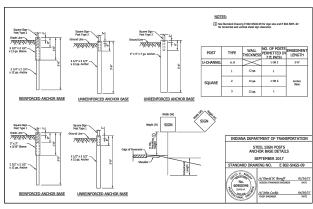
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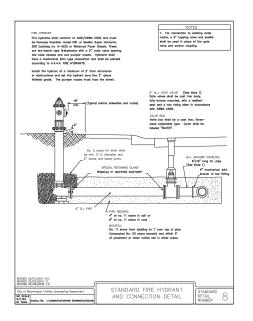
60% PLANS NOT FOR CONSTRUCTION

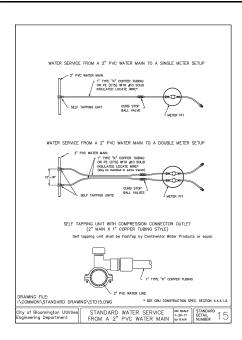
SHEET 802

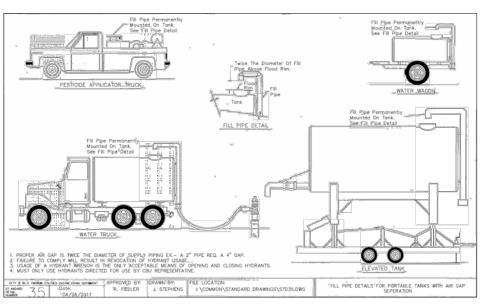














ഗ **DETAIL**(BLOOMINGTON HOPEWELL

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CONSTRUCTION 60% PLANS NOT FOR

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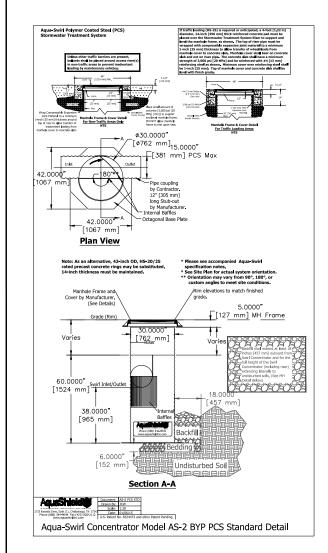
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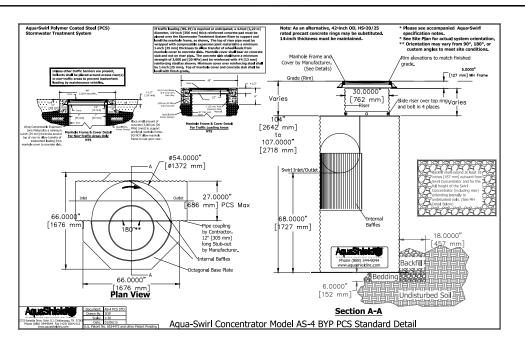
60% PLANS

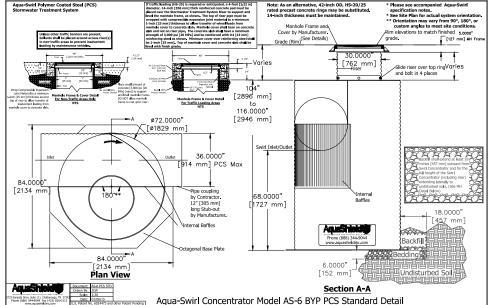
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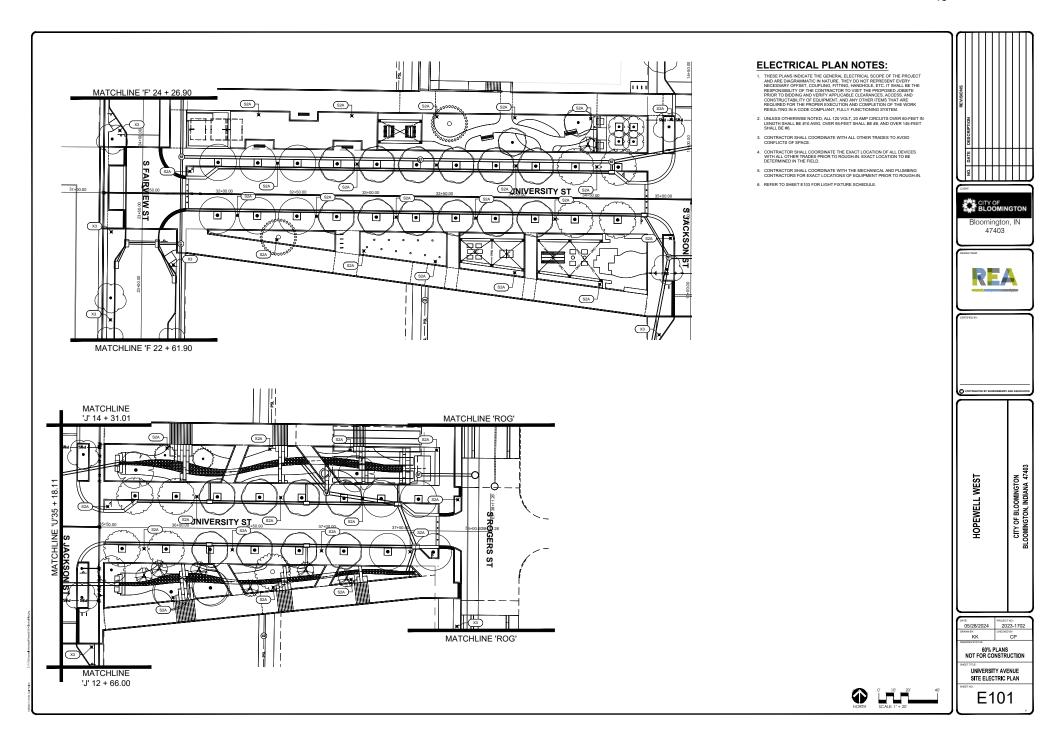


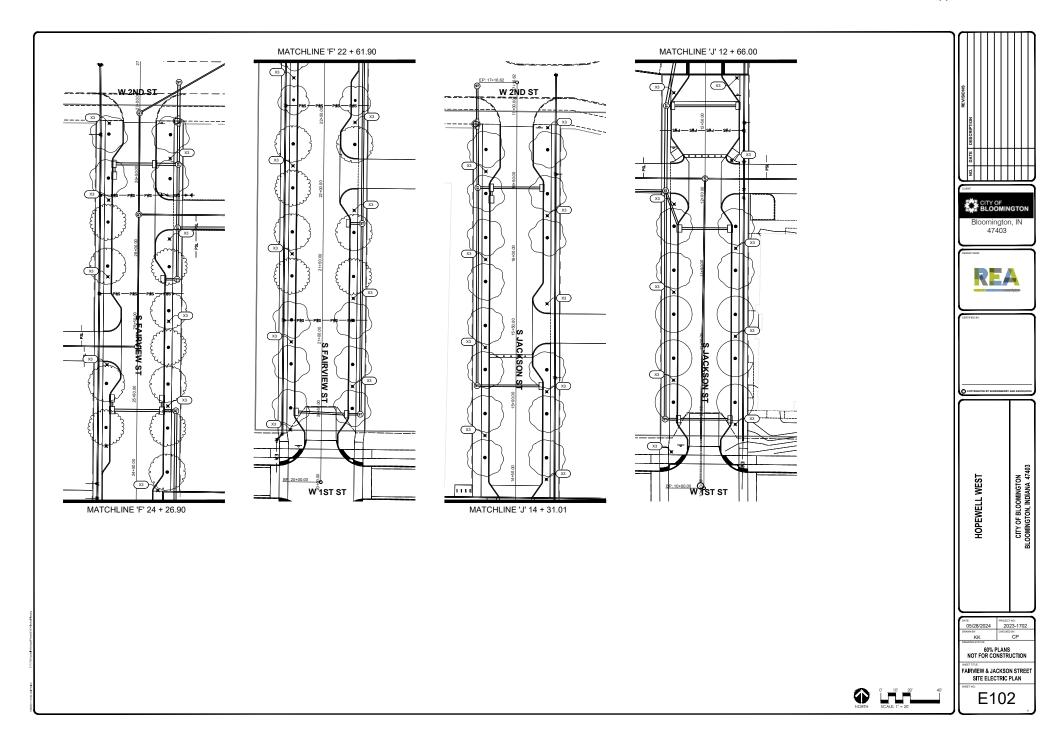


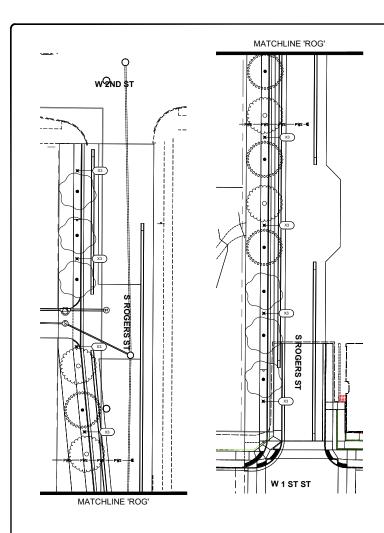


ORY PATH : R:\Active\Roomington, Ciry Of\Hopewel West\Design\CAD\Plans
-- and any uscurer prima is done

SHEET 804







	Light Fixture Schedule							
Туре	Manufacturer	Model No.	Lamp Type	Voltage	Mounting	Description	Illustration (See Note 1)	Notes
'S2A'	Landscape Forms Duke	ARTHRECAWE (2 @ 190" - 65" 18 - 19" mounting ht) ARCA2 (pole) Traditional, 12" pole	LED / 4000 K		Pole Pole	Area IgRing assembly, Four (2) LED luminaires with wide flood optics and mounting arms. 14 round bapered aluminum pole. Wet location listed. Mate black finish. 12' mounting height, direct bury pole. Black finish.		7

Notes:

I Pickures are shown as a visual aid only and may not depict specific attributes listed in this schedule.

2 Emergency backup: Minimum of 90 minute nurines for emergency operation.

3. 101/ LED dimmer wider. Provide manufacture-approved composible dimmer where applicable (refer to lighting plans for dimmer location(s)).

4. E.V reserve phasing dimming. Provide manufacture-approved composible dimmer where applicable (refer to lighting plans for dimmer location(s)).

4. E.V reserve phasing dimming. Provide manufacture-approved composible dimmer where applicable (refer to lighting plans for dimmer location(s)).

6. Provide all comercions are equired by the Manufacturer. Virgin exact quantities of all system components prior to ordering. Install all system components per manufacturer's requirements.

7. Coordinate Bithimming of luminatre(s) in field with Engineer/Architect.





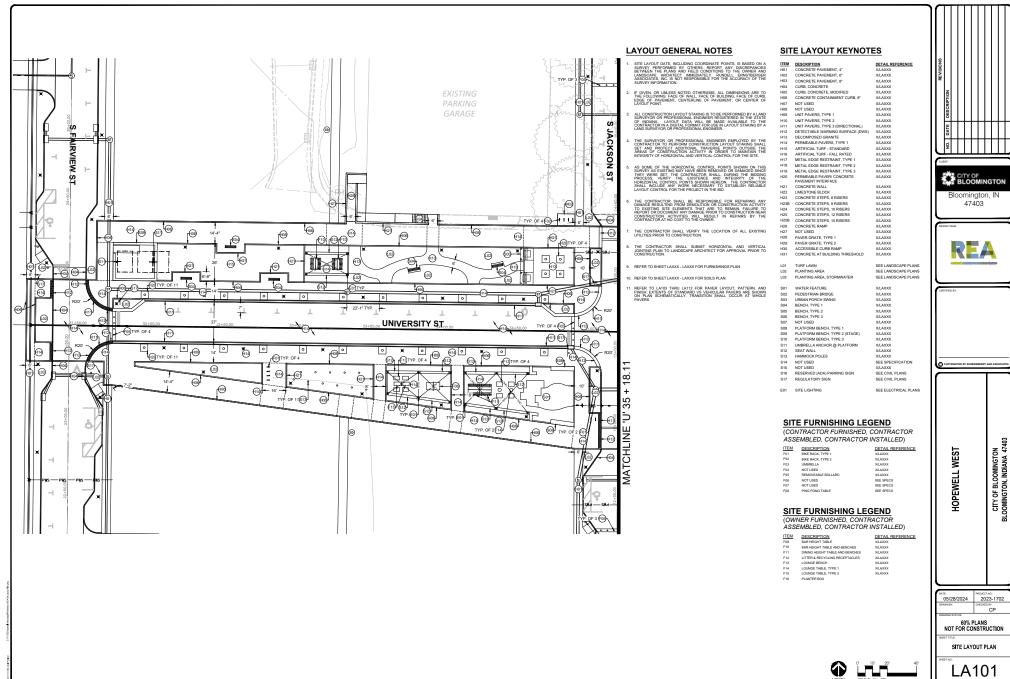


HOPEWELL WEST

CITY OF BLOOMINGTON BLOOMINGTON, INDIANA 47403









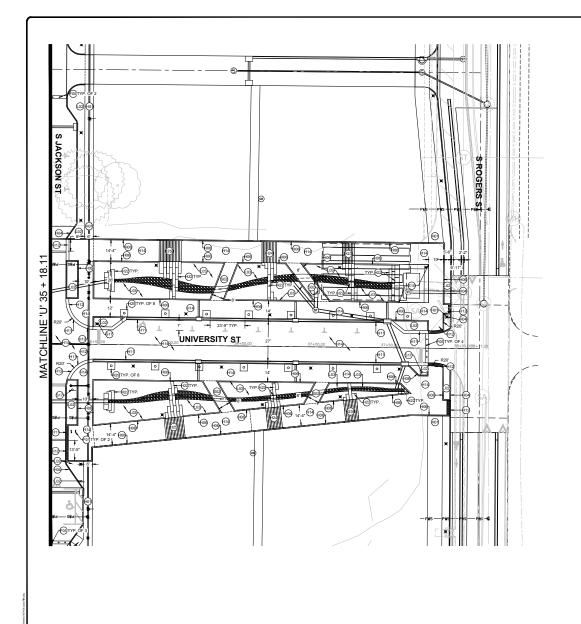








DETAIL REFERENCE



LAYOUT GENERAL NOTES

- SITE LAYOUT DATE, INCLUDING COORDINATE POINTS, IS BASED ON A SURVEY PERFORMED BY OTHERS, REPORT ANY DISCREPANCIES BETWEEN THE PLANS AND FIELD CONDITIONS TO THE OWNER AND LANDSCAPE ARCHITECT IMMEDIATELY, RUNDELL EINSTREAM
- IF GIVEN, OR UNLESS NOTED OTHERWISE, ALL DIMENSIONS ARE TO THE FOLLOWING: FACE OF WALL, FACE OF BUILDING, FACE OF CURB, EDGE OF PAYEMENT, CENTERLINE OF PAVEMENT, OR CENTER OF LAYOUT POINT.
- 3. ALL CONSTRUCTION LAYOUT STAKING IS TO BE PERFORMED BY A LAND SURVEYOR OR PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF INDIANAL LAYOUT DATA WILL BE AMDE AVAILABLE TO THE CONTRACTOR IN A DIGITAL FORMAT FOR USE IN LAYOUT STAKING BY A LAND SURVEYOR OR PROFESSIONAL ENGINEER.
- 4. THE SURVEYOR OR PROFESSIONAL ENGINEER EMPLOYED BY THE CONTRACTOR TO PERFORM CONSTRUCTION LAYOUT STAKING SHALL SET AND PROTECT ADDITIONAL TRAVERSE POINTS OUTSIDE THE AREAS OF CONSTRUCTION ACTIVITY IN ORDER TO MAINTAIN THE INTEGRITY OF HORIZONTAL AND VERTICAL CONTROL FOR THE SITE.
- S. AS SOME OF THE HORIZOPTAL CONTROL POINTS SHOWN ON THE SURVEY AS EXISTING MAN HAVE BEEN REMOVED ON BOMBON SINCE THEY WERE SET, THE CONTRACTOR SHALL DURING THE BROWNE PROCESS, VERRY THE EXISTENCE AND INTEGRITY OF THE SHALL INCLUDE ANY WORK RECESSARY TO ESTABLISH RELIMBLE LAYOUT CONTROL FOR THE PROJECT IN THE BIT.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRING ANY DAMAGE RESULTING FROM DEBOLITION OF CONSTRUCTION ACTIVITY REPORT OR DOCUMENT ANY DAMAGE PRIOR TO CONSTRUCTION NEAR CONSTRUCTION ACTIVITIES WILL RESULT IN REPAIRS BY THE CONTRACTOR AT NO COST TO THE OWNER.
- THE CONTRACTOR SHALL VERIFY THE LOCATION OF ALL EXISTING UTILITIES PRIOR TO CONSTRUCTION.
- 9. REFER TO SHEET LAXXX LAXXX FOR FURNISHINGS PLAN.
- 11. REFER TO LA103 THRU LA112 FOR PAVER LAYOUT, PATTERN, AND FINISH. EXTENTS OF STANDARD VS VEHICULAR PAVERS ARE SHOWN ON PLAN SCHEMATICALLY. TRANSITION SHALL OCCUR AT WHOLE

SITE LAYOUT KEYNOTES

ITEM	DESCRIPTION	DETAIL REFEREI
H01	CONCRETE PAVEMENT, 4"	X/LAXXX
H02	CONCRETE PAVEMENT, 6"	X/LAXXX
H03	CONCRETE PAVEMENT, 8"	X/LAXXX
H04	CURB, CONCRETE	X/LAXXX
H05	CURB, CONCRETE, MODIFIED	X/LAXXX
H06	CONCRETE CONTAINMENT CURB, 8*	X/LAXXX
H07	NOT USED	X/LAXXX
H08	NOT USED	X/LAXXX
H09	UNIT PAVERS, TYPE 1	X/LAXXX
H10	UNIT PAVERS, TYPE 2	X/LAXXX
H11	UNIT PAVERS, TYPE 3 (DIRECTIONAL)	X/LAXXX
H12	DETECTABLE WARNING SURFACE (DWS)	X/LAXXX
H13	DECOMPOSED GRANITE	X/LAXXX
H14	PERMEABLE PAVERS, TYPE 1	X/LAXXX
H15	ARTIFICIAL TURF - STANDARD	X/LAXXX
H16	ARTIFICIAL TURF - FALL RATED	X/LAXXX
H17	METAL EDGE RESTRAINT, TYPE 1	X/LAXXX
H18	METAL EDGE RESTRAINT, TYPE 2	X/LAXXX
H19	METAL EDGE RESTRAINT, TYPE 3	X/LAXXX
H20	PERMEABLE PAVER/ CONCRETE PAVEMENT INTERFACE	X/LAXXX
H21	CONCRETE WALL	X/LAXXX
H22	LIMESTONE BLOCK	X/LAXXX
H23	CONCRETE STEPS, 8 RISERS	X/LAXXX
H23B	CONCRETE STEPS, 9 RISERS	X/LAXXX
H24	CONCRETE STEPS, 10 RISERS	X/LAXXX
H25	CONCRETE STEPS, 12 RISERS	X/LAXXX
H25B	CONCRETE STEPS, 15 RISERS	X/LAXXX
H26	CONCRETE RAMP	X/LAXXX
H27	NOT USED	X/LAXXX
H28	PAVER GRATE, TYPE 1	X/LAXXX

PAVER GRATE, TYPE 2	X/LAXXX
ACCESSIBLE CURB RAMP	X/LAXXX
CONCRETE AT BUILDING THRESHOLD	X/LAXXX
TURF LAWN	SEE LANDSCAPE PLAI
PLANTING AREA	SEE LANDSCAPE PLAI
PLANTING AREA, STORMWATER	SEE LANDSCAPE PLAI
WATER FEATURE	X/LAXXX
PEDESTRIAN BRIDGE	X/LAXXX
URBAN PORCH SWING	X/LAXXX
BENCH, TYPE 1	X/LAXXX
BENCH, TYPE 2	X/LAXXX
BENCH, TYPE 3	X/LAXXX
NOT USED	X/LAXXX
PLATFORM BENCH, TYPE 1	X/LAXXX
PLATFORM BENCH, TYPE 2 (STAGE)	X/LAXXX
PLATFORM BENCH, TYPE 3	X/LAXXX
UMBRELLA ANCHOR @ PLATFORM	X/LAXXX
SEAT WALL	X/LAXXX
HAMMOCK POLES	X/LAXXX
NOT USED	SEE SPECIFICATION
NOT USED	X/LAXXX
RESERVED (ADA) PARKING SIGN	SEE CIVIL PLANS
REGULATORY SIGN	SEE CIVIL PLANS

SITE FURNISHING LEGEND (CONTRACTOR FURNISHED, CONTRACTOR

E01 SITE LIGHTING

ASSEMBLED, CONTRACTOR INSTALLED) EFERENCE

SEE ELECTRICAL PLANS

DESCRIPTION	DETAIL RE
BIKE RACK, TYPE 1	XILAXXX
BIKE RACK, TYPE 2	X/LAXXX
UMBRELLA	XILAXXX
NOT USED	X/LAXXX
REMOVEABLE BOLLARD	XILAXXX
NOT USED	SEE SPECS
NOT USED	SEE SPECS

SITE FURNISHING LEGEND

(OWNER FURNISHED, CONTRACTOR

ITEM	DESCRIPTION	DETAIL REFERE
F09	BAR HEIGHT TABLE	XILAXXX
F10	BAR HEIGHT TABLE AND BENCHES	XILAXXX
F11	DINING HEIGHT TABLE AND BENCHES	XILAXXX
F12	LITTER & RECYCLING RECEPTACLES	XILAXXX
F13	LOUNGE BENCH	XILAXXX
F14	LOUNGE TABLE, TYPE 1	XILAXXX
F15	LOUNGE TABLE, TYPE 2	XILAXXX
F16	PLANTER BOX	









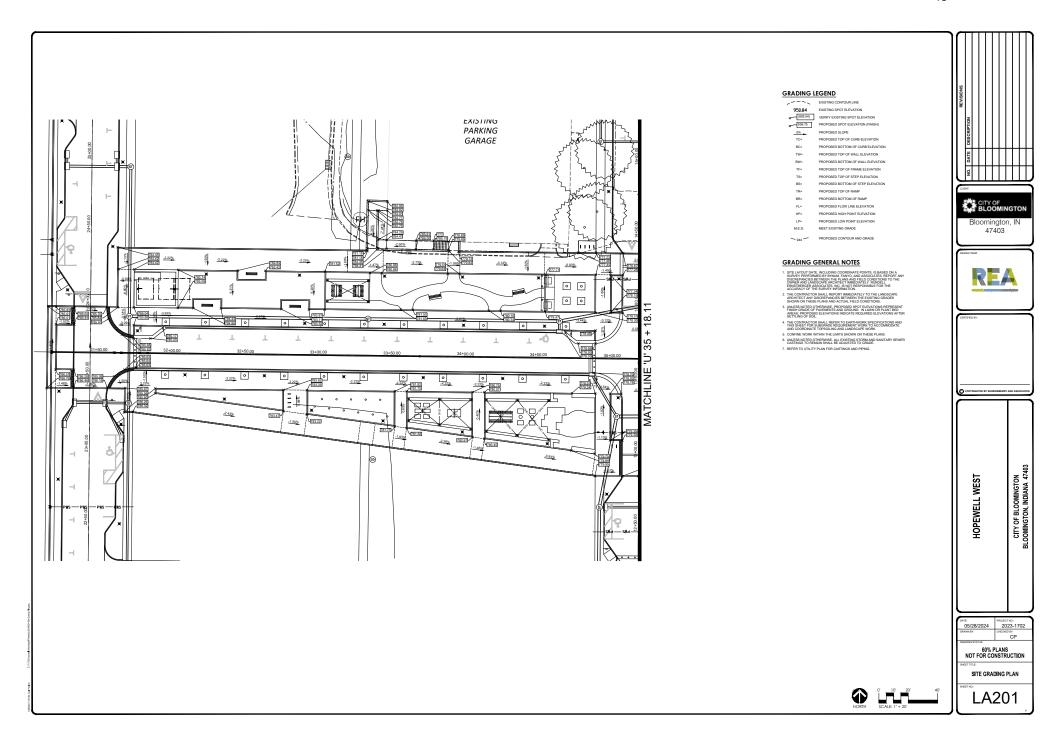


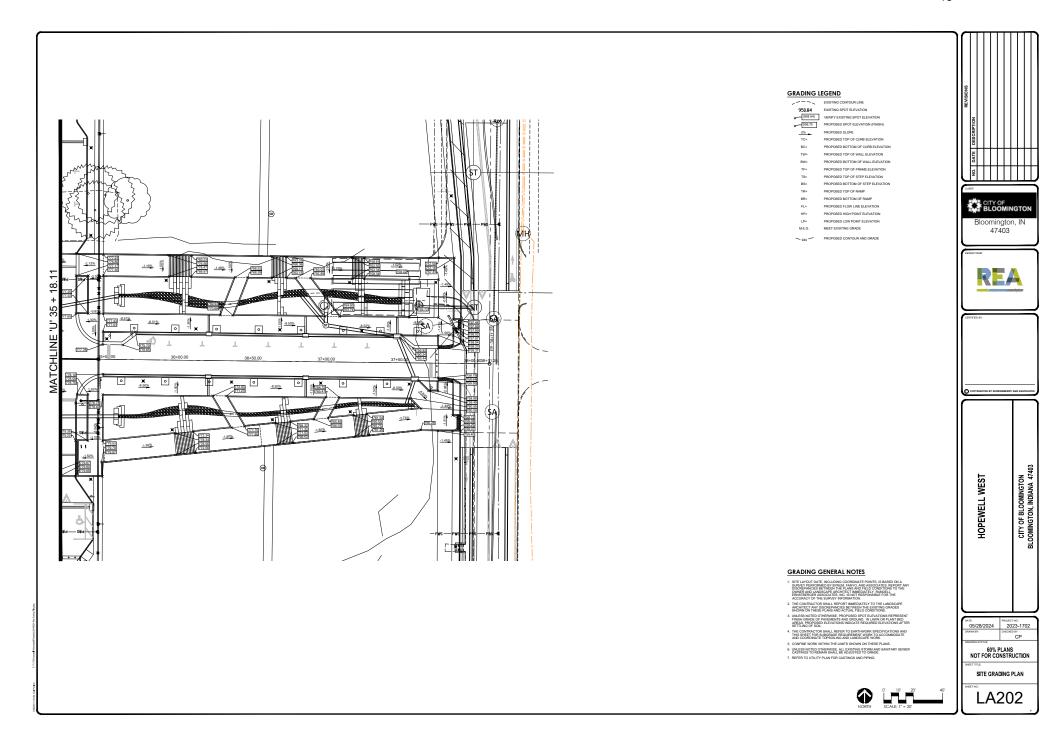
CITY OF BLOOMINGTON BLOOMINGTON, INDIANA 47403

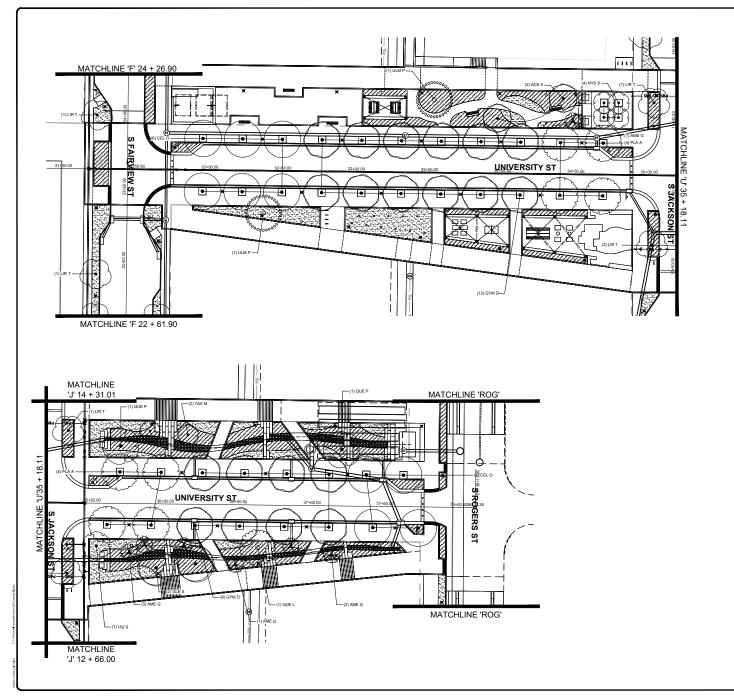
HOPEWELL WEST

05/28/2024 2023-1702 60% PLANS NOT FOR CONSTRUCTION SITE LAYOUT PLAN









PLANTING NOTES:

- NOTIFY OWNER/ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES BETWEEN THE SPECIFICATIONS AND DRAWINGS, PRIOR TO BID DATE, AND/OR PRIOR TO CONSTRUCTION.
- LANDSCAPE ARCHITECT AND OWNER TO INSPECT ALL PLANT LOCATIONS AND PLANT BED EDGES PRIOR TO INSTALLATION. THE LANDSCAPE ARCHITECT RESERVES THE RIGHT TO ADJUST PLANT LOCATIONS ON-SITE. ON-SITE ADJUSTMENTS WILL BE REQUIRED.
- 3. PLANT COUNTS NOICATED ON DRAWINGS ARE FOR LANDSCAPE
 ARCHITECTOWNER'S USE ONLY CONTRACTOR SHALL MAKE OWN PLANT
 COMMITTED AND CONTRACTOR SHALL MAKE OWN PLANT
 COMMITTED AND CONTRACTOR SHALL MAKE OWN PLANT
 BY THE LANDSCAPE ARCHITECT CONTRACTOR TO VERRY SED
 MEASUREMENTS AND INSTALL APPORPHATE OUNTRACTOR SHALL ACCOUNT FOR
 ALL SLOPES IN AMTERIAL QUARTIT CACULATIONS.
- IN CASE OF DISCREPANCIES BETWEEN THE PLANS AND PLANT LIST, THE PLAN SHALL DICTATE IF IN QUESTION CONTACT THE LANDSCAPE ARCHITECT
- DO NOT MAKE SUBSTITUTIONS. IF SPECIFIED LANDSCAPE MATERIAL IS NOT AVAILABLE, SUBMIT PROOF OF NON-AVAILABILITY TO LANDSCAPE ARCHITECT TOGETHER WITH A PROPOSAL FOR USE OF EQUIVALENT MATERIAL LANDSCAPE ARCHITECT RESERVES THE RIGHT TO DETERMINE MATERIAL EQUIVALENCY.
- CONTRACTOR SHALL INSTALL PLANTING SOIL IN ALL PROPOSED PLANT BED AREAS, AND TOPSOIL IN ALL LAWN AND DISTURBED AREAS, UNLESS NOTED OTHERWISE.
- 7. THE EARTHWORK CONTRACTOR SHALL COORDINATE THE PLACEMENT AND GRADING OF SUBSOIL TO ACCOMMODATE TOPSOIL. REFER TO SOIL DETAILS AND SPECIFICATIONS.
- 8. PROTECT STRUCTURES, UTILITIES, SIDEWALKS, PAVEMENTS, AND OTHER FACILITIES AND EASTING EXTERIOR PLANTS FROM DAMAGE CAUSED BY PLANTING OPERATIONS. RECONDITION AND SOD ALL AREAS DISTURBED BY CONSTRUCTION ACTIVITIES THAT ARE NOT TO RECEIVE OTHER SURFACE TREATMENT (PRESERVED, RENOVATED AREAS, MULCH, GROUNDCOVER, ETC.).
- PLANT AND ALL OTHER MATERIALS TO BE STORED ON-SITE WILL BE PLACED WHERE THEY WILL NOT CONFLICT WITH CONSTRUCTION OPERATIONS AND AS DIRECTED BY THE OWNER.
- 10. CONTRACTOR SHALL NOTIFY LANDSCAPE ARCHITECT IN WRITING PRIOR TO BID DATE OF ANY PLANTS THAT HE/SHE FELES MAY NOT SURVIVE TRANSPLANTING OPERATIONS OR IN LOCATIONS NOTED.
- 11. PLANT BEDS TO RECEIVE MIN. 3" OF SHREDDED HARDWOOD MULCH (UNLESS OTHERWISE NOTED) SEE SPECIFICATIONS. THE USE OF COMPOST MIX IN PERENNAL, ORNAMENTAL GRASS, AND GROUND COVER BEDS IS ACCEPTABLE UPON WRITTEN APPROVAL BY OWNER/ARCHITECT.
- PROVIDE SHOVEL-CUT SPADE EDGE ADJACENT TO ALL PLANTING BED AREAS NOT BORDERED BY CONCRETE OR OTHER EDGING. SEE SPADE EDGE DETAIL.
- 13. ALL TREE PITS TO BE DUG IN ADVANCE OF PLANTING AND EACH TREE PIT TO BE TESTED FOR ADEQUATE DRAWAGE SEE SPECIFICATIONS FOR MISTRUCTIONS ON HOW TO ADMINISTER THE TEST FOR RATE OF PERCOLATION. PROVIDE DRY WELL IF DETERMINED RECESSARY BY LANGSCAPE ARCHITECT. SEE SPECIFICATIONS AND DETAIL.
- 14. USE SOIL EXCAVATED FROM TREE PLANTING HOLE AS BACKFILL FOR TREE PLANTINGS WHEN POSSIBLE.
- 15. AN APPROVED PRE-EMERGENT HERBICIDE SHALL BE APPLIED IN ALL PLANTING AND GROUNDCOVER BEDS AT RATES SPECIFIED BY THE MANUFACTURER FOR EACH VARIETY OF PLANT.
- 16. SEE PLANTING SCHEDULE AND LANDSCAPE DETAILS FOR FURTHER REQUIREMENTS.
- 17. REFER TO DEMOLITION PLANS, DETAILS, AND SPECIFICATIONS FOR EROSION CONTROL MEASURES INCLUDING TEMPORARY SEEDING AND ADDITIONAL REQUIREMENTS.
- 18. REFER TO EXISTING CONDITION PLAN FOR ALL EXISTING UNDERGROUND PIPE AND IRRIGATION LOCATIONS.

REFER TO SHEET LA304 FOR PLANTING SCHEDULE AND DETAILS.







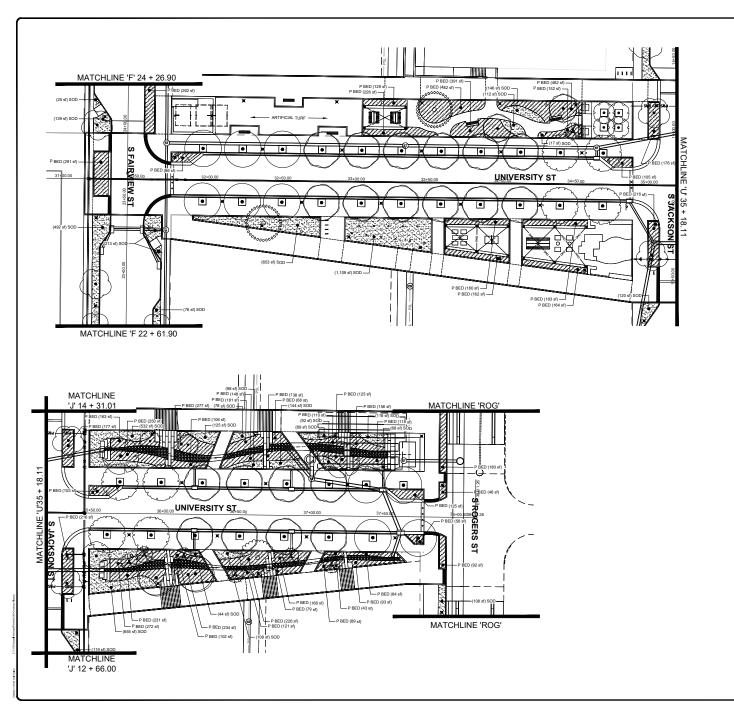
COPPROVIED BY SHEWSBEERLY AND ASSOCIATES

CITY OF BLOOMINGTON BLOOMINGTON, INDIANA 47403

HOPEWELL WEST

SOS/28/2024 2023-1702
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PLANTING NOTES:

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NO DATE DESCRIPTION





) COPPRONTED BY SHIRKNESSERY AND ASSOCIATES

CITY OF BLOOMINGTON BLOOMINGTON, INDIANA 47403

HOPEWELL WEST

ATC: 05/28/2024 PROJECT NO.: 05/28/2024 2023-1702
RARIN BP: CHECKED BY: CP

RAWMONG STATULE: 60% PLANS

NOT FOR CONSTRUCTION

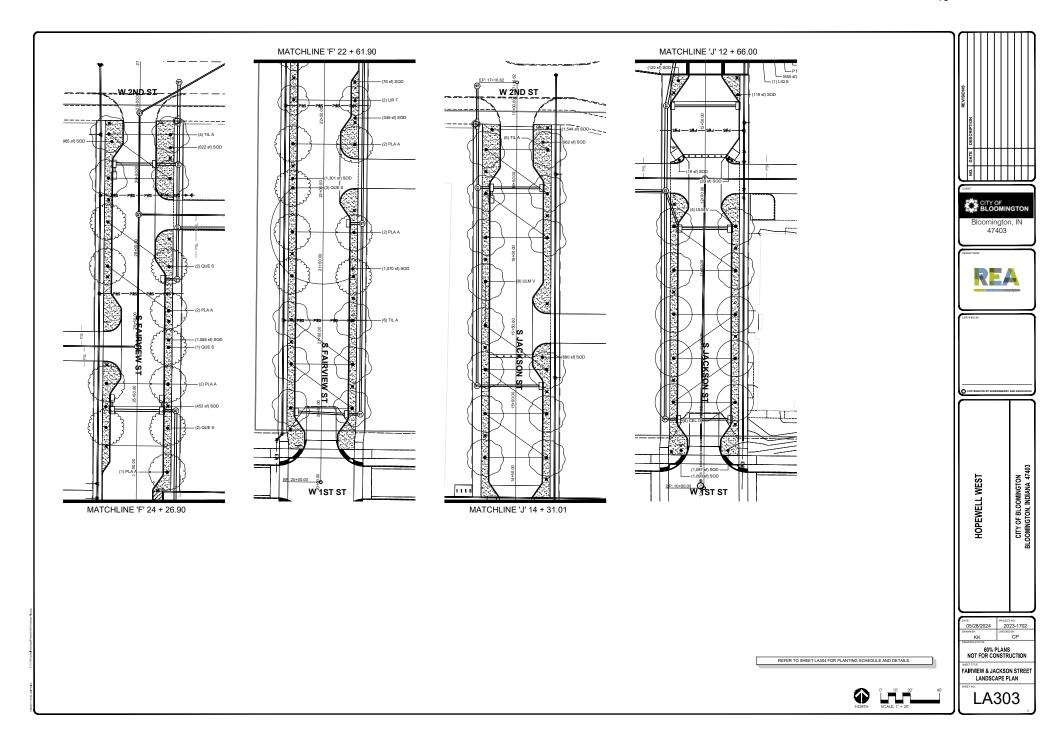
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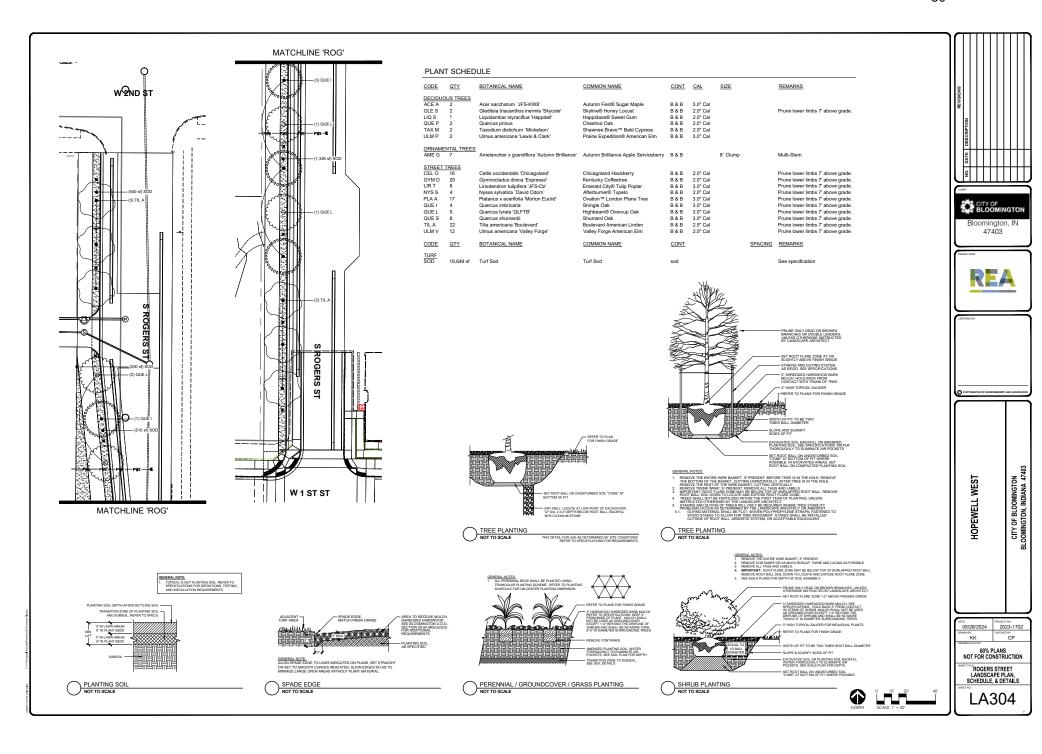
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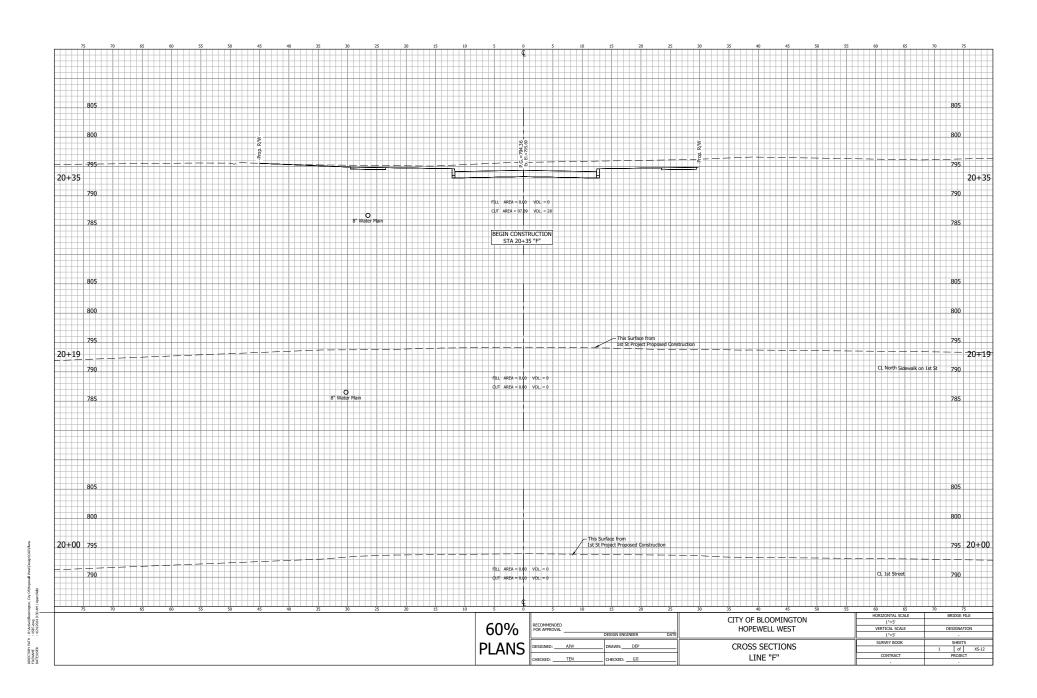
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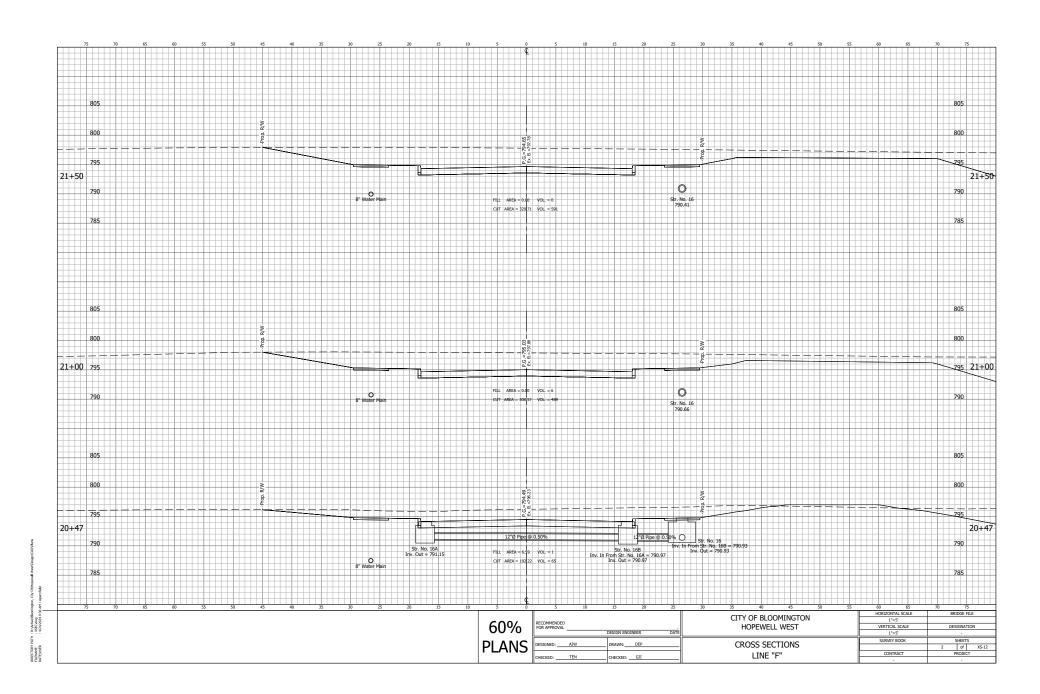


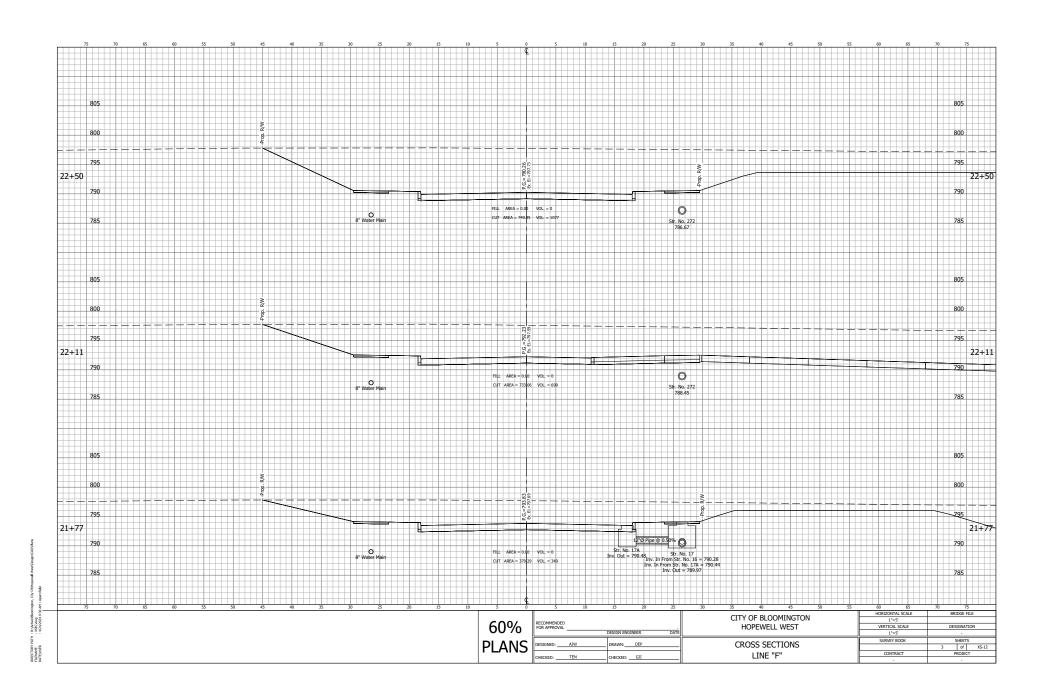
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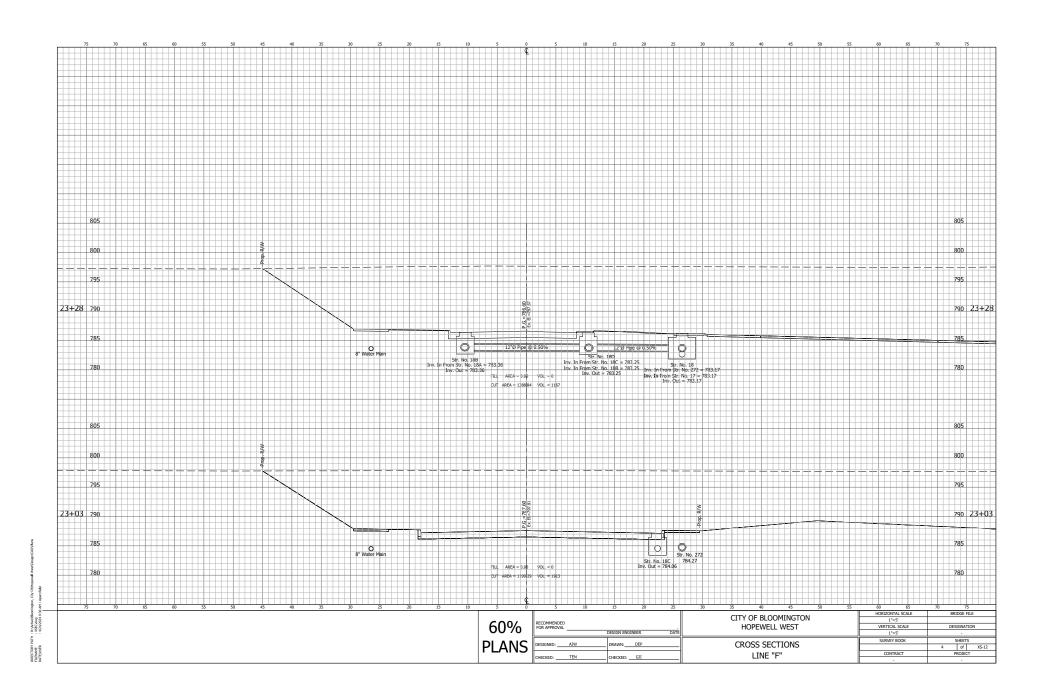


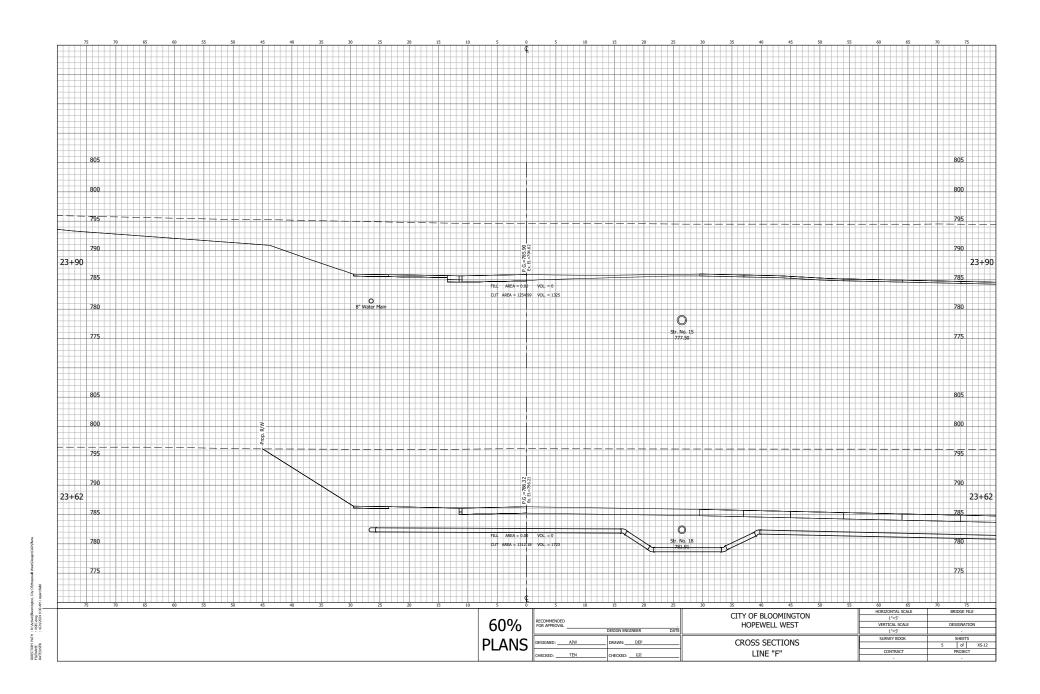


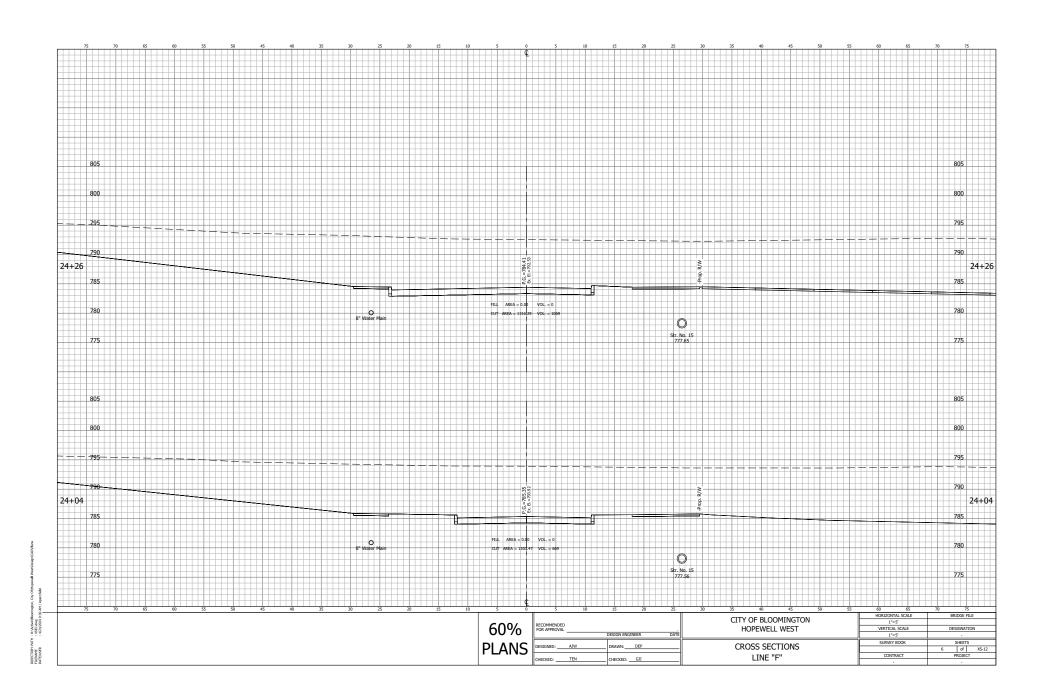


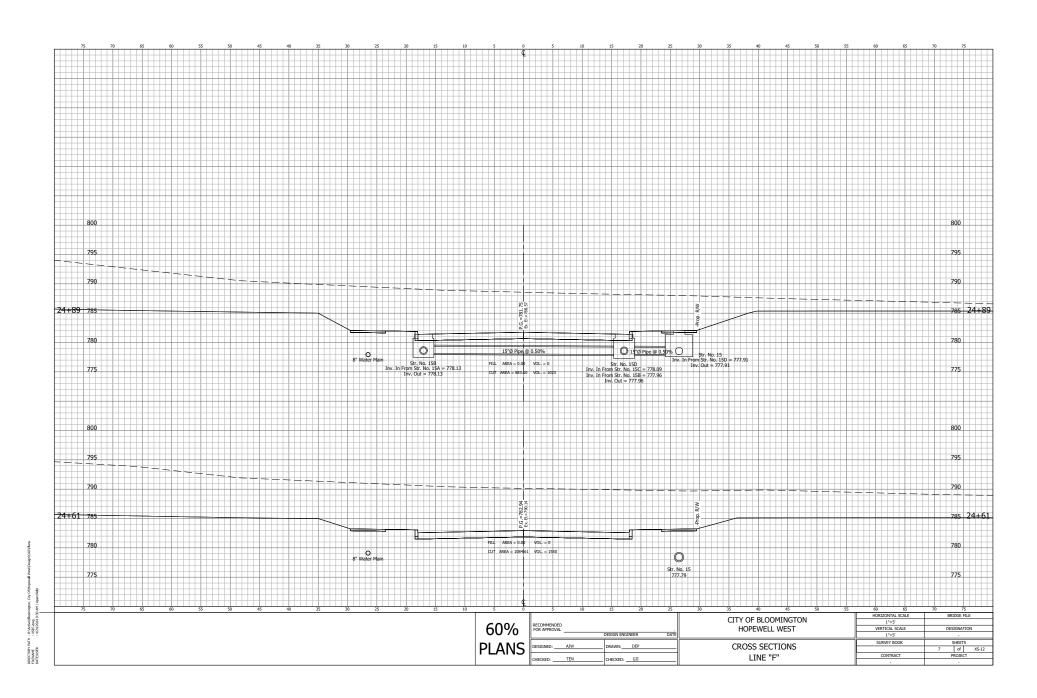


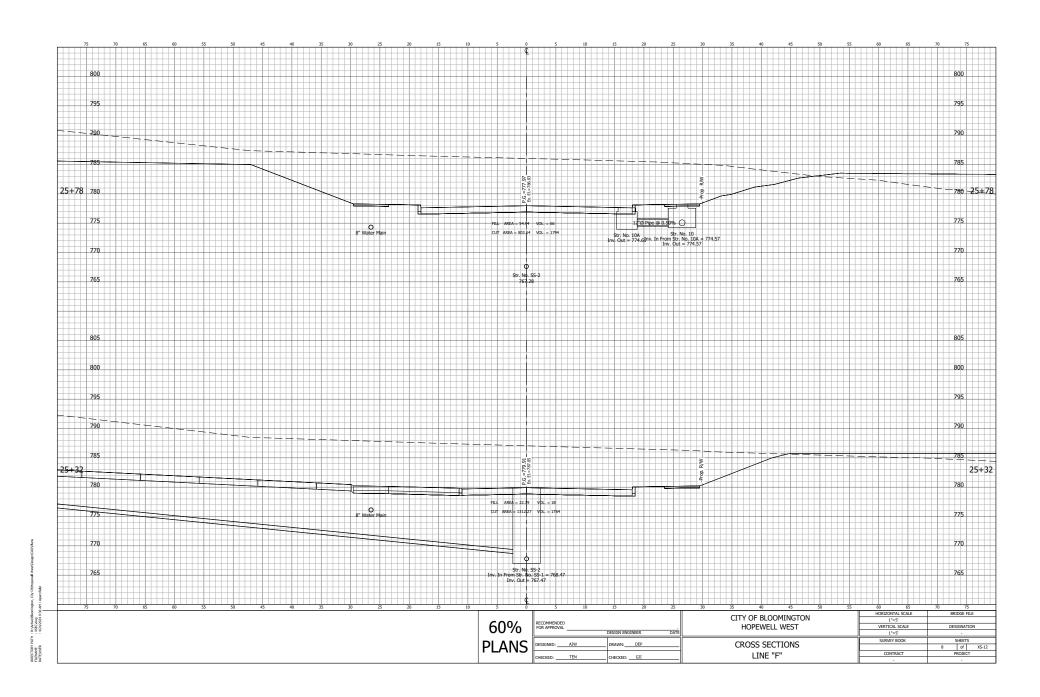


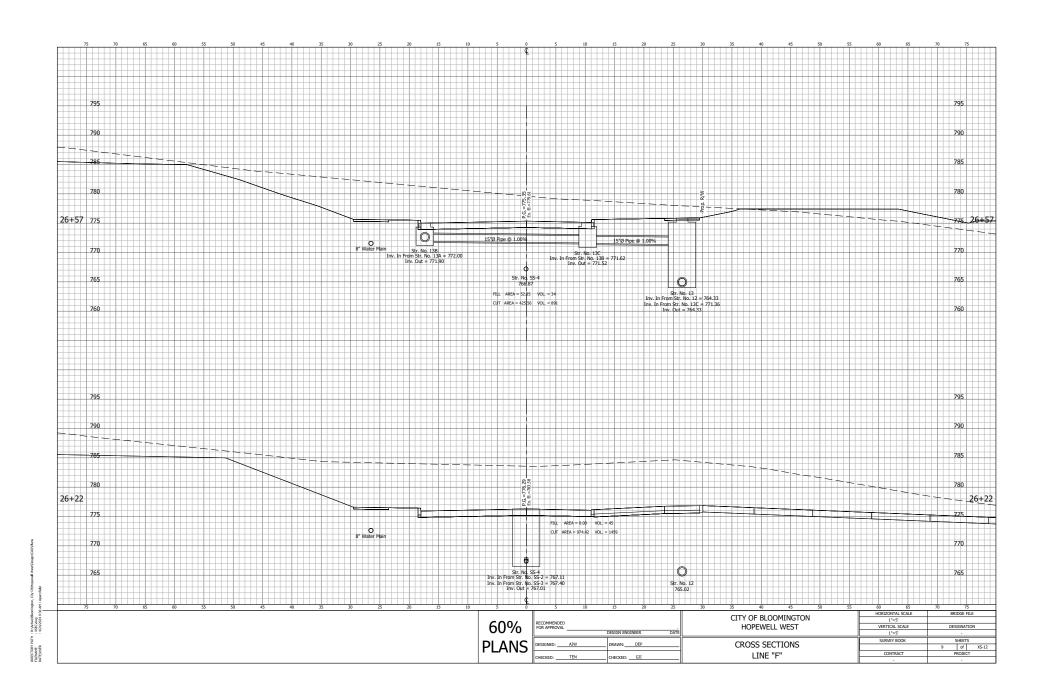


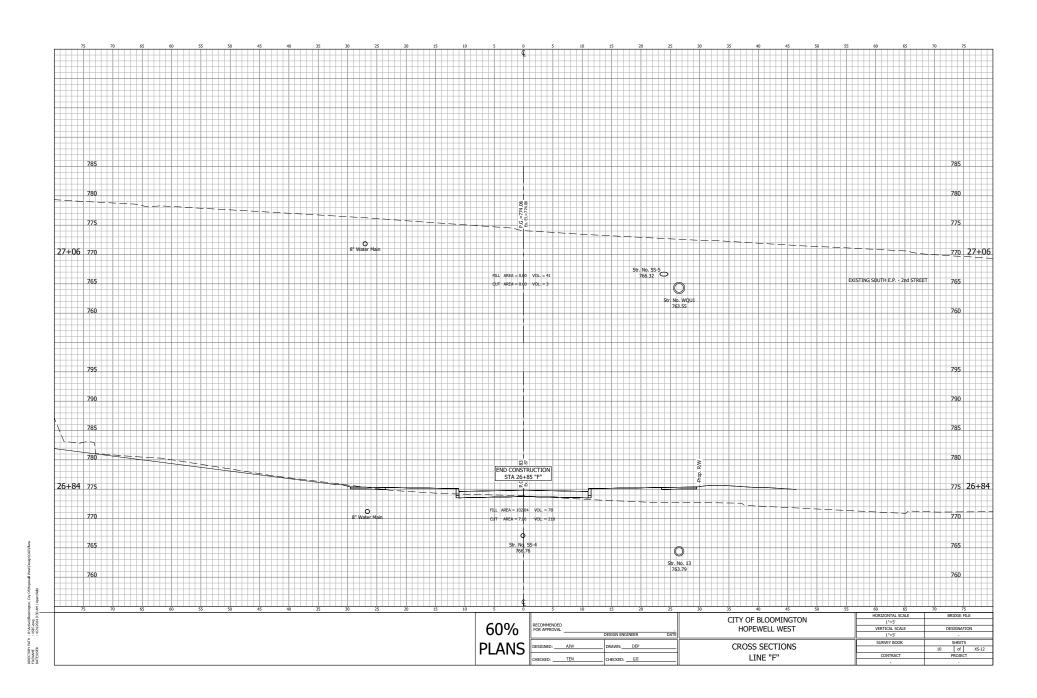


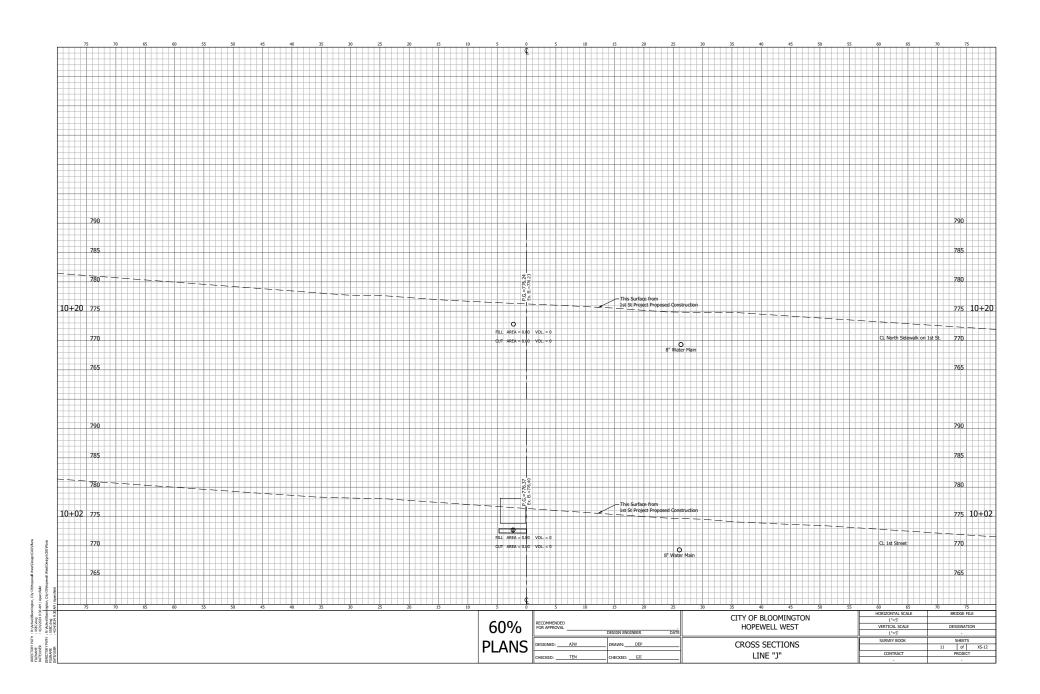


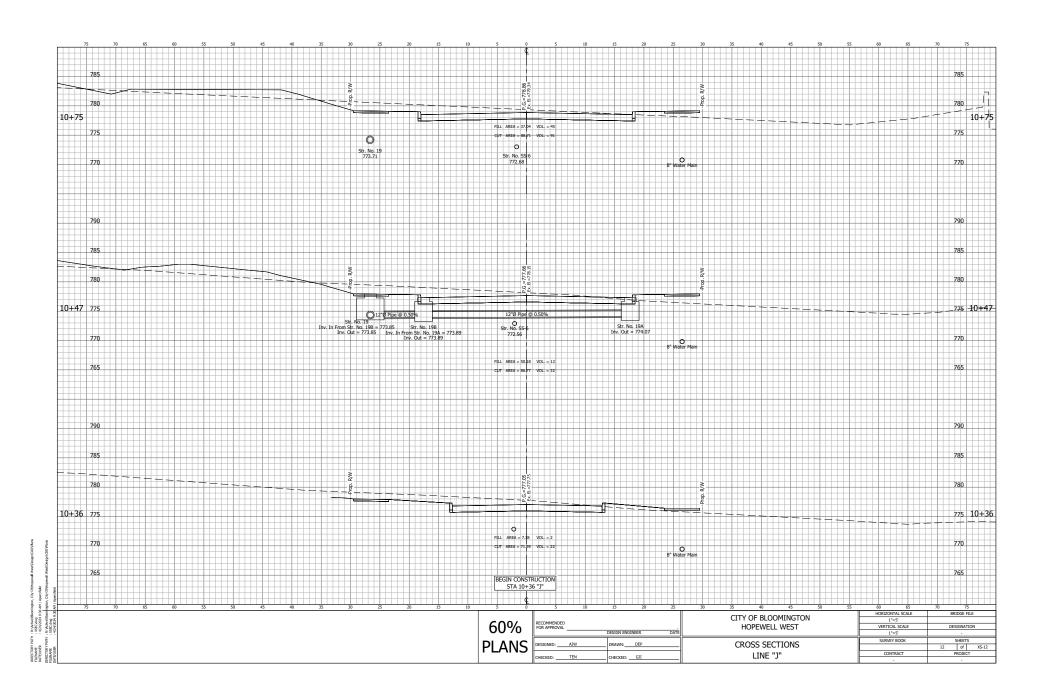


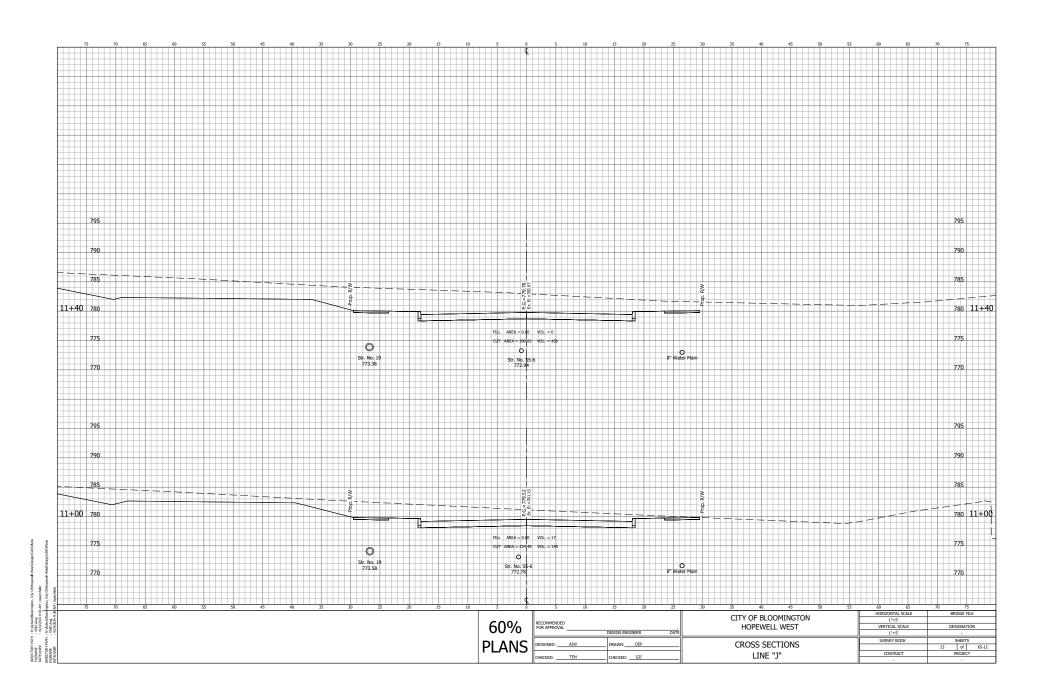


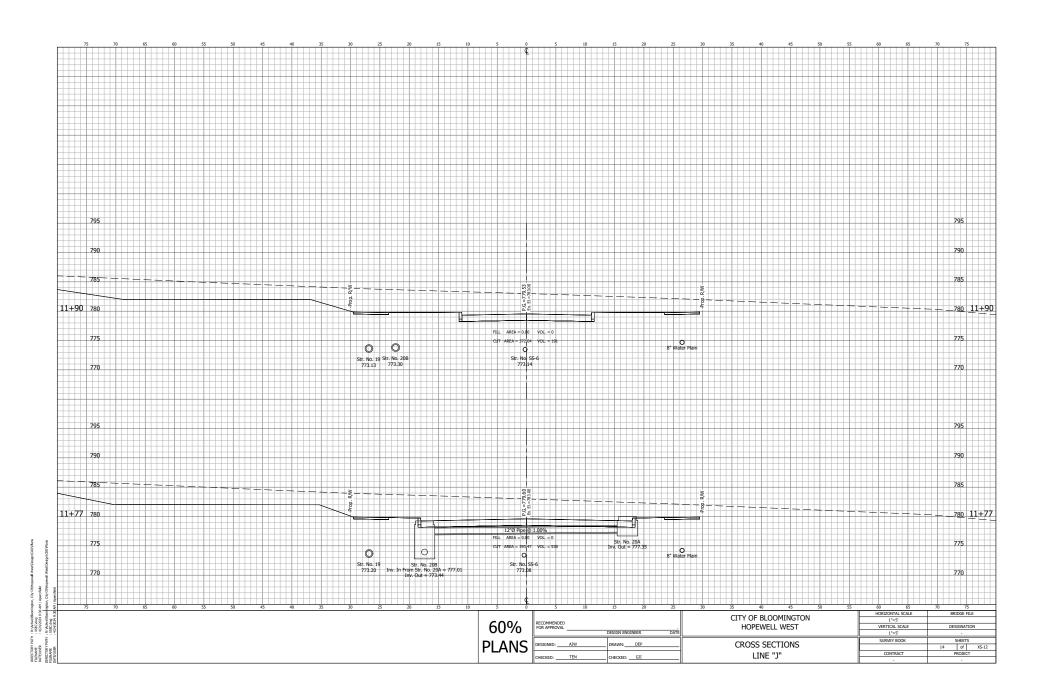


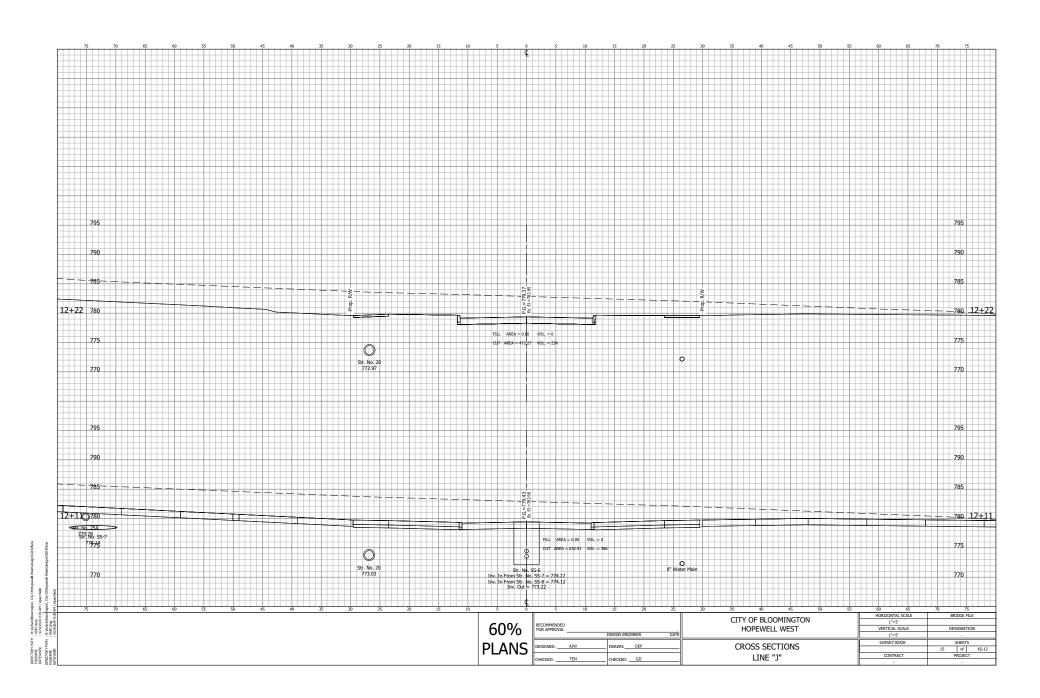


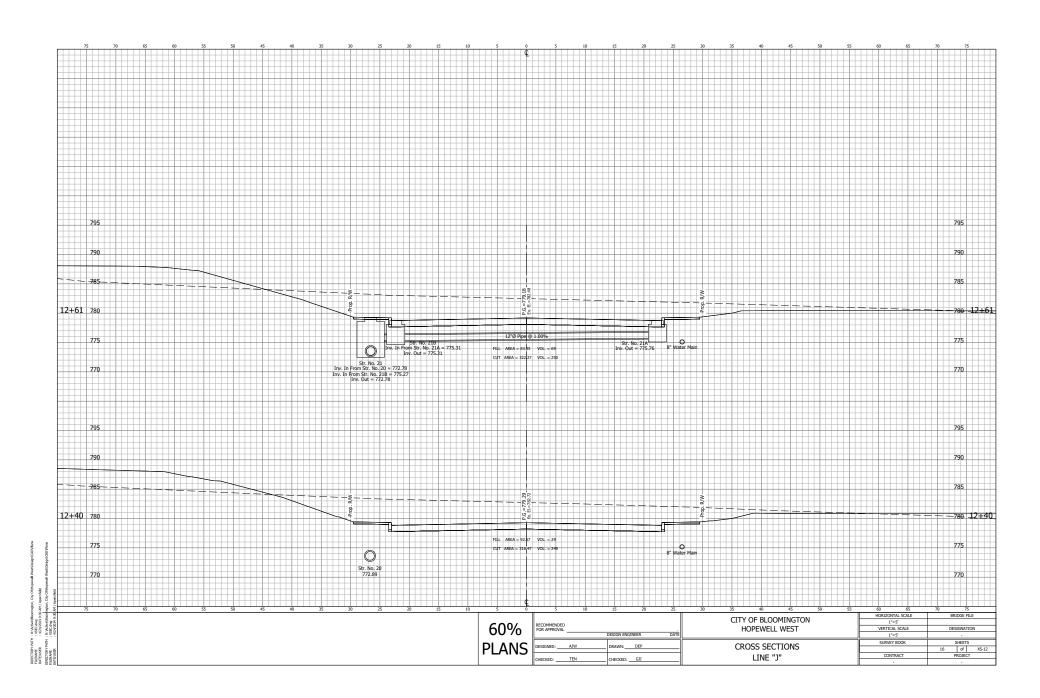


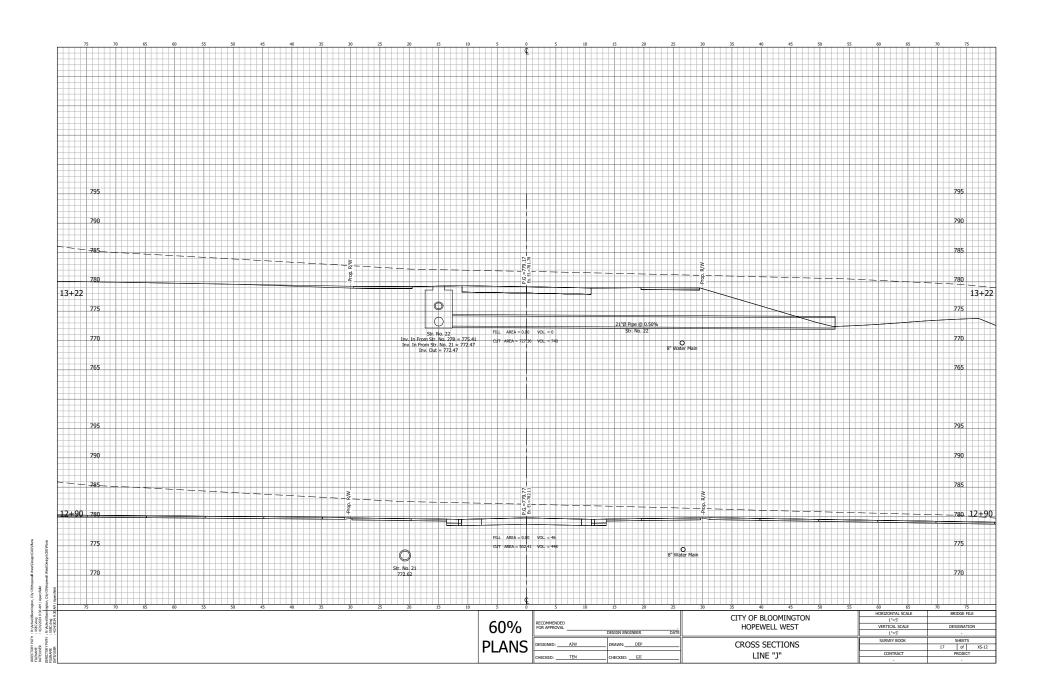


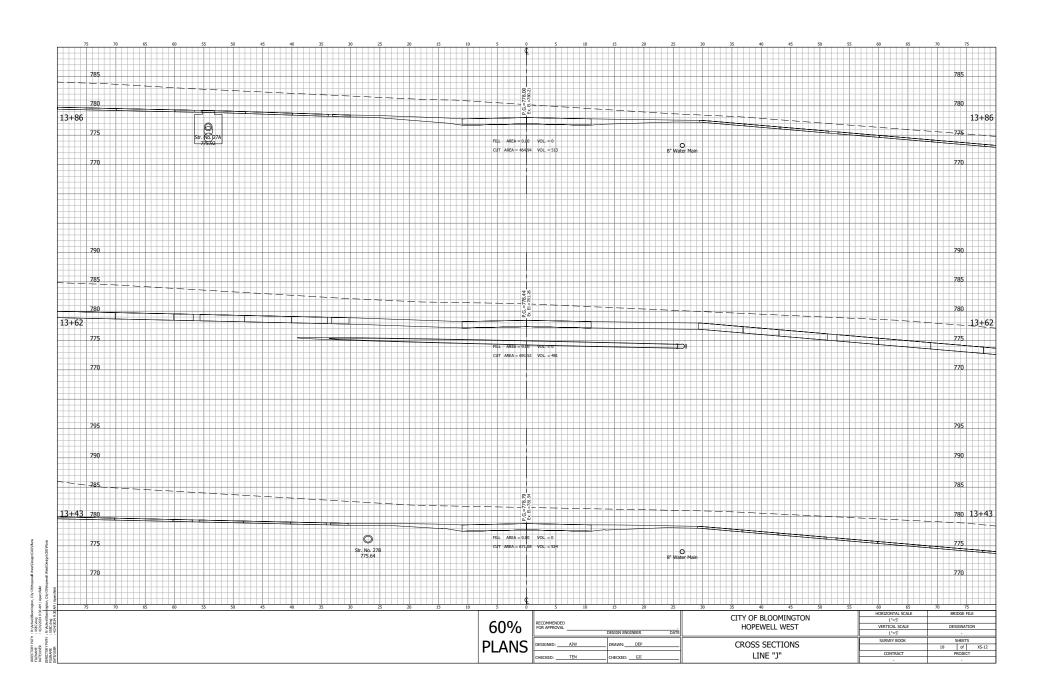


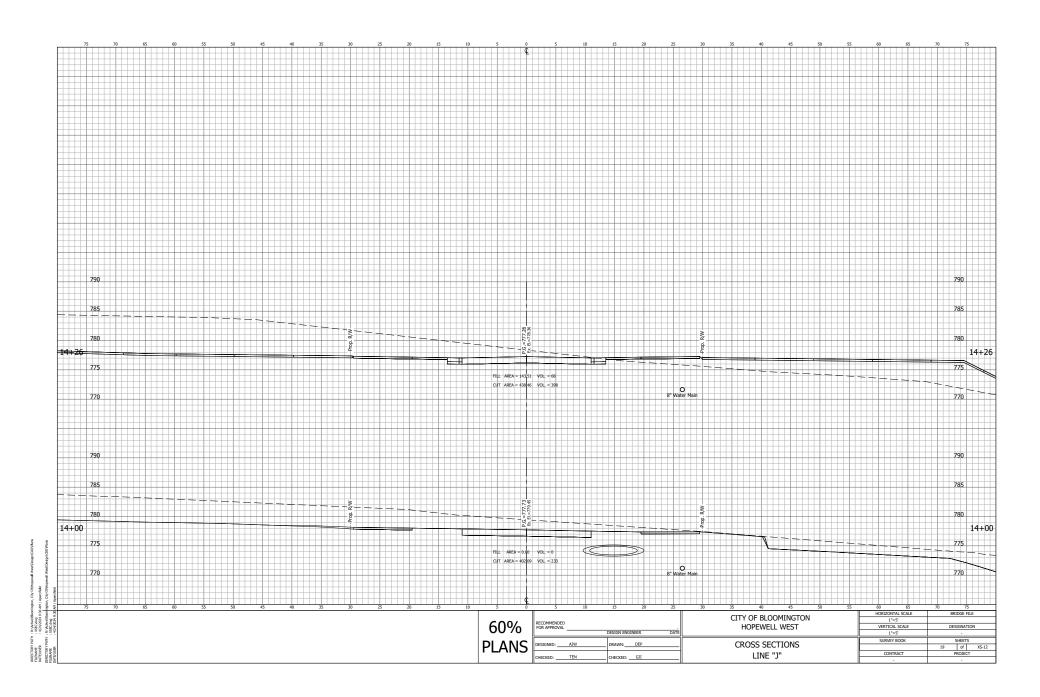


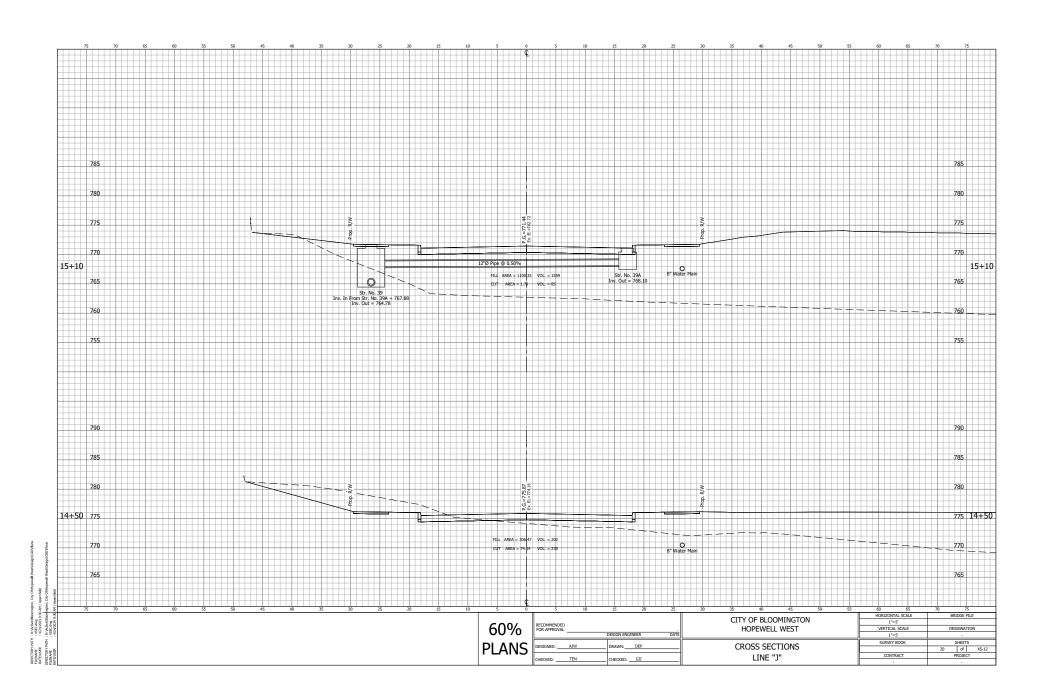


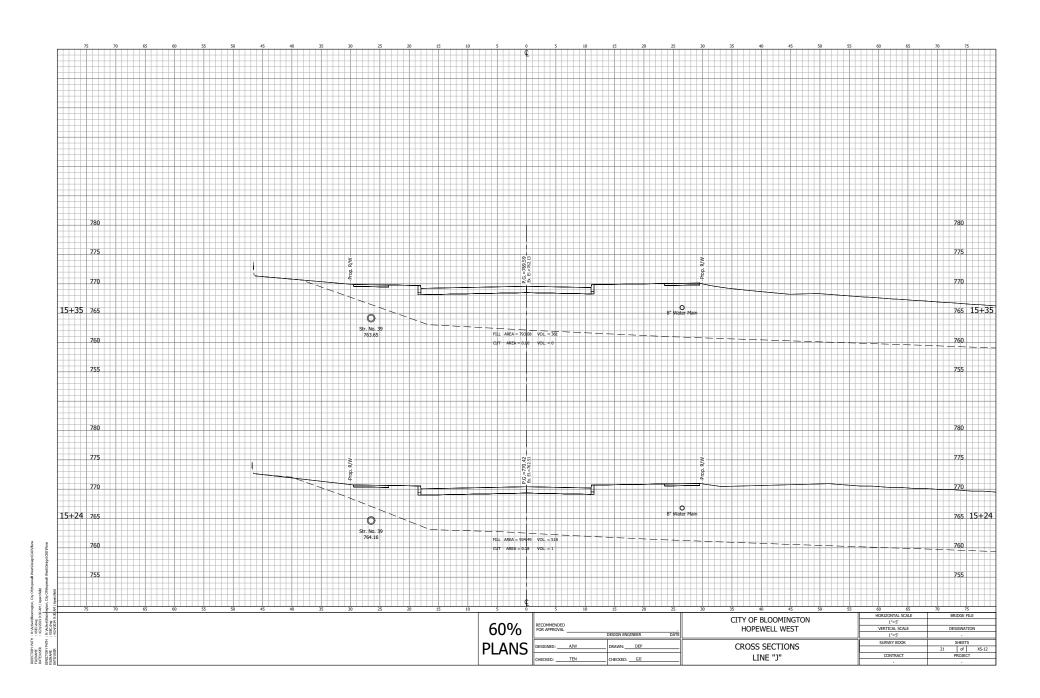


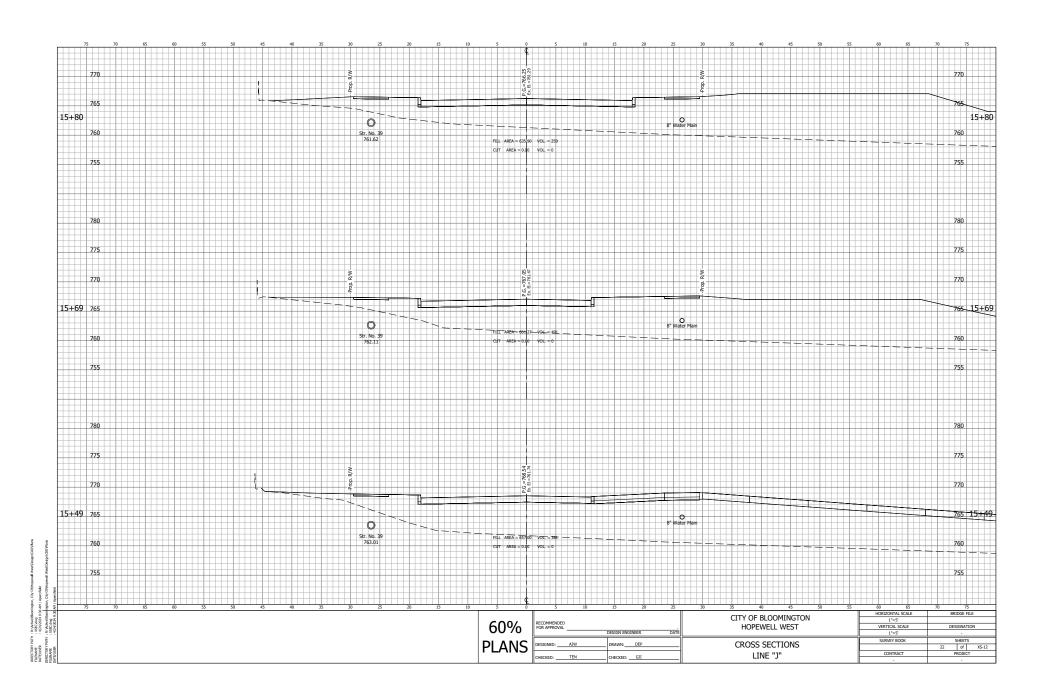


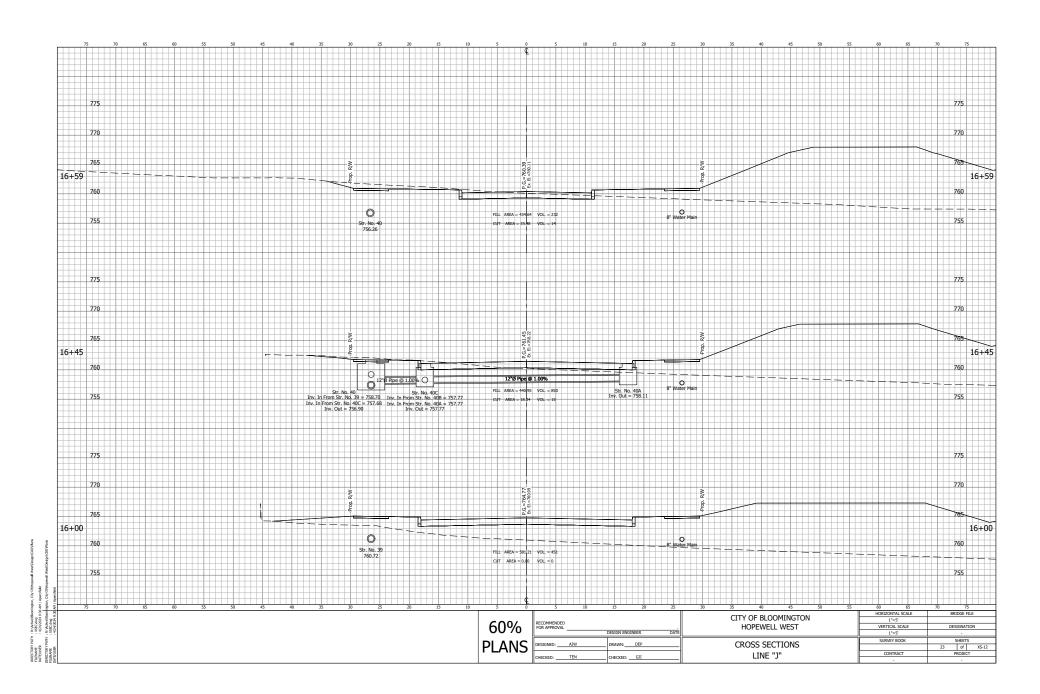


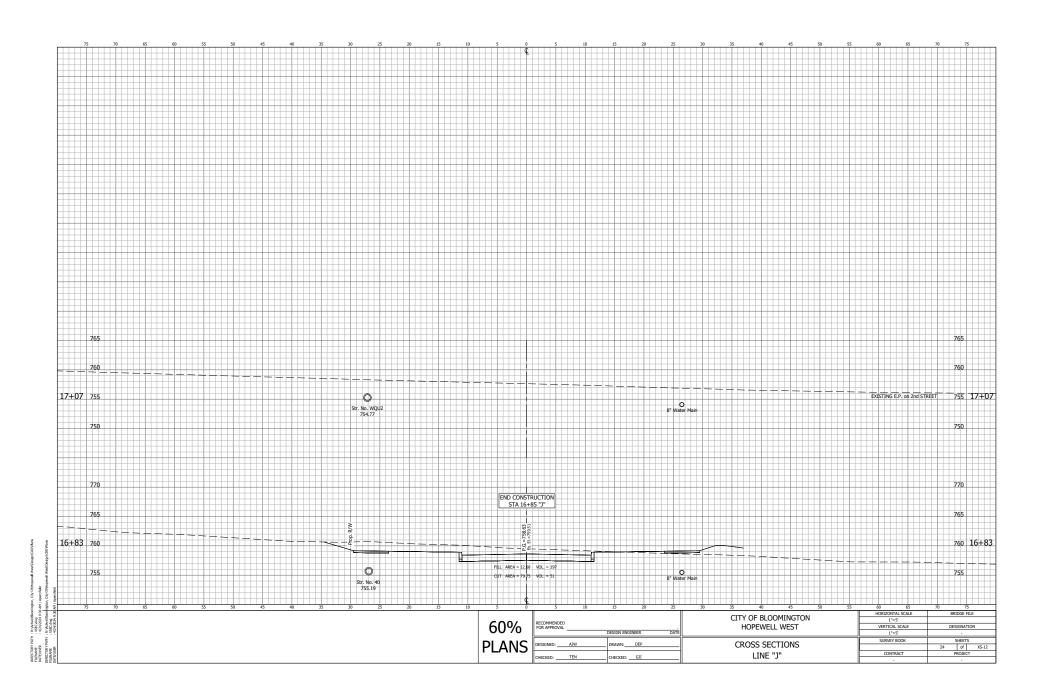


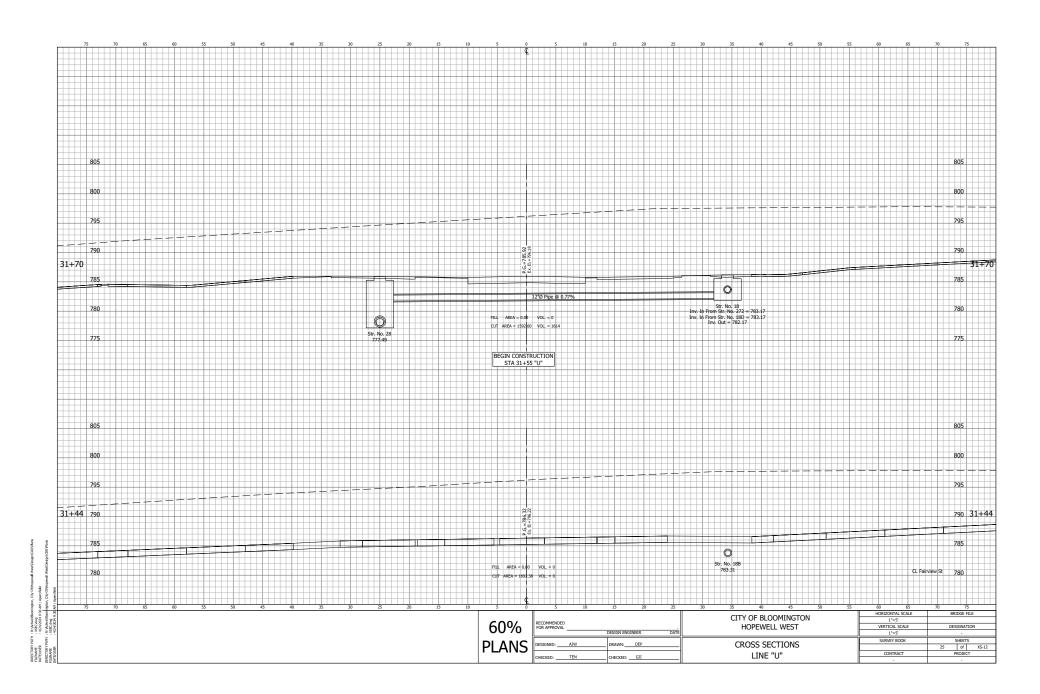


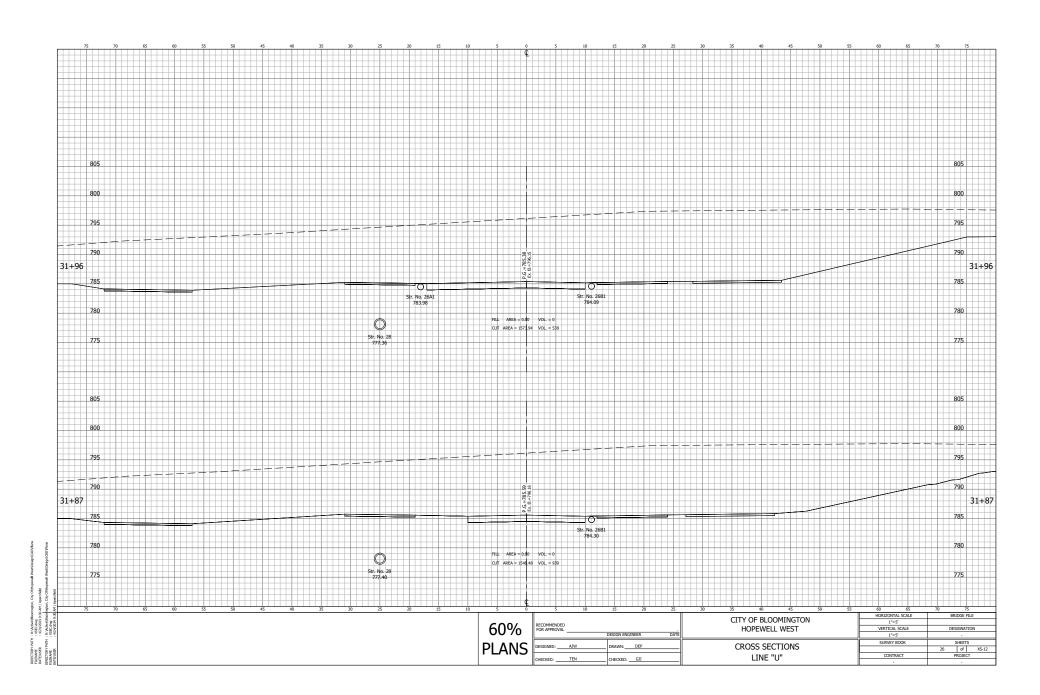


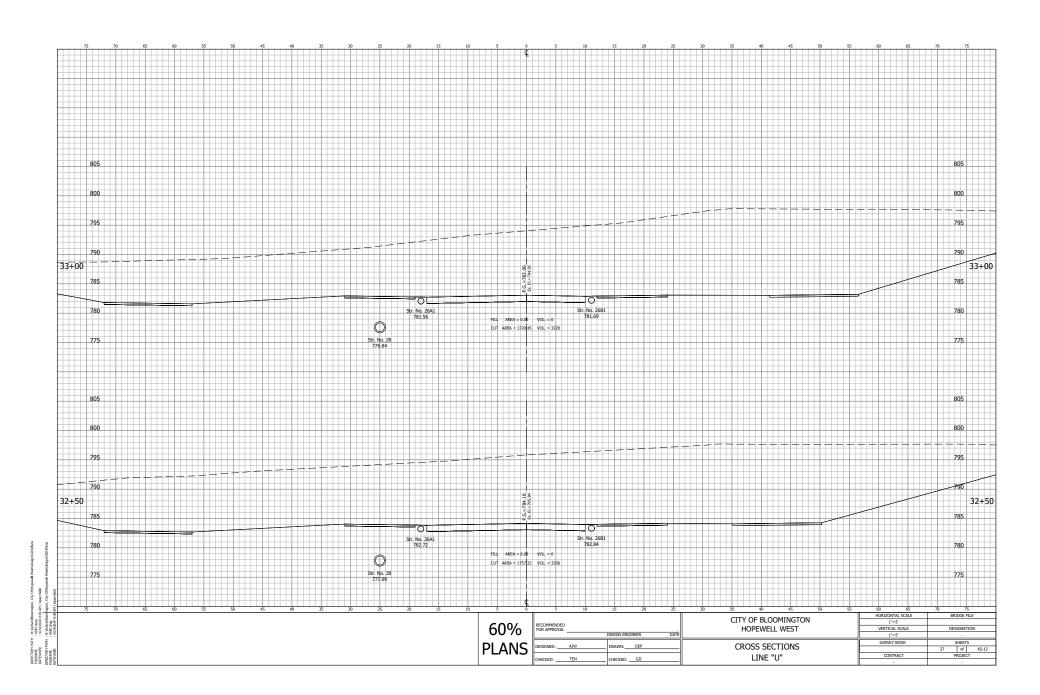


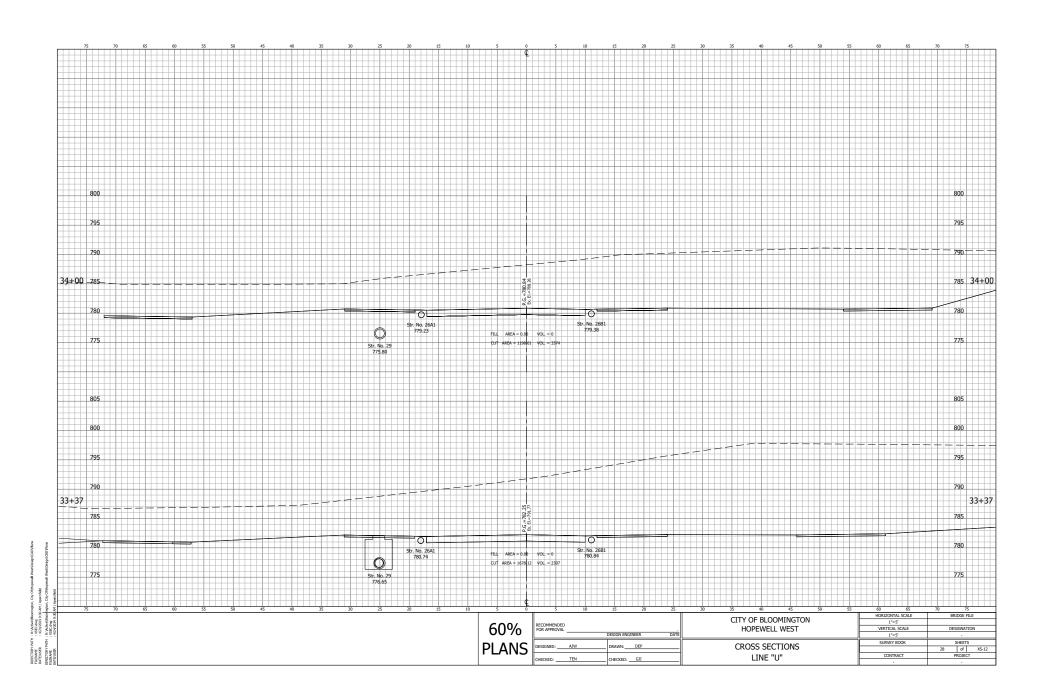


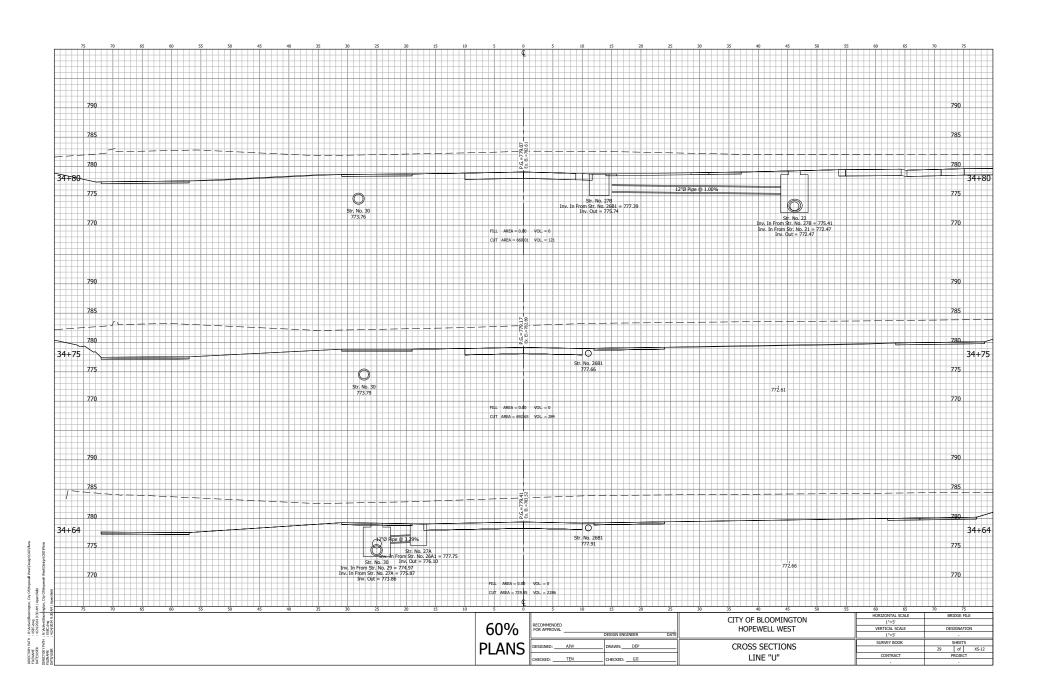


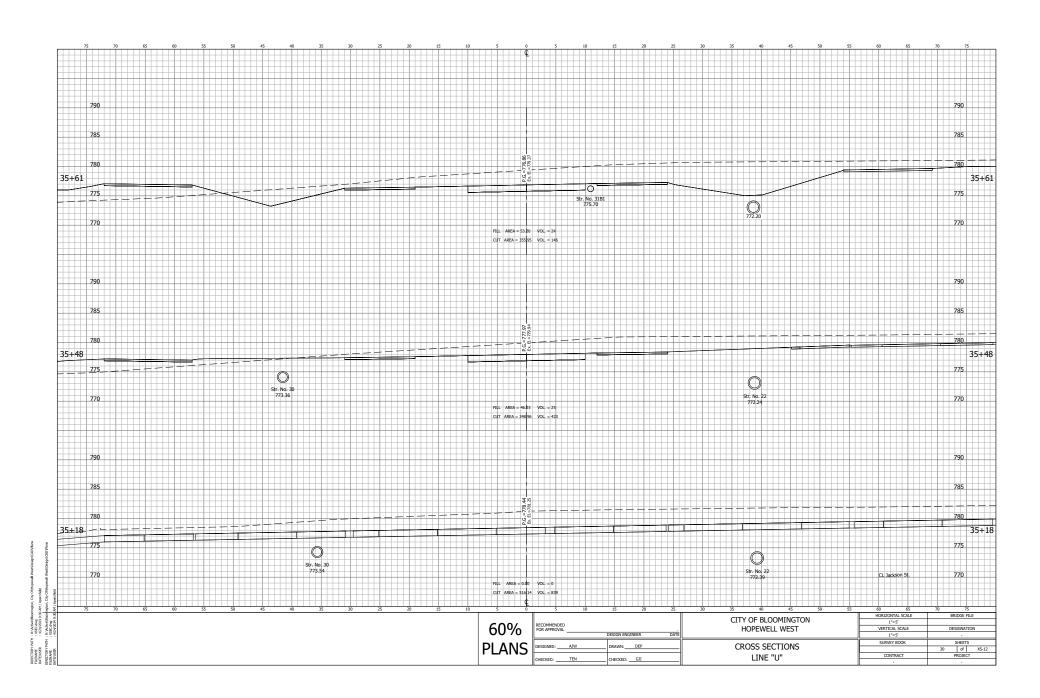


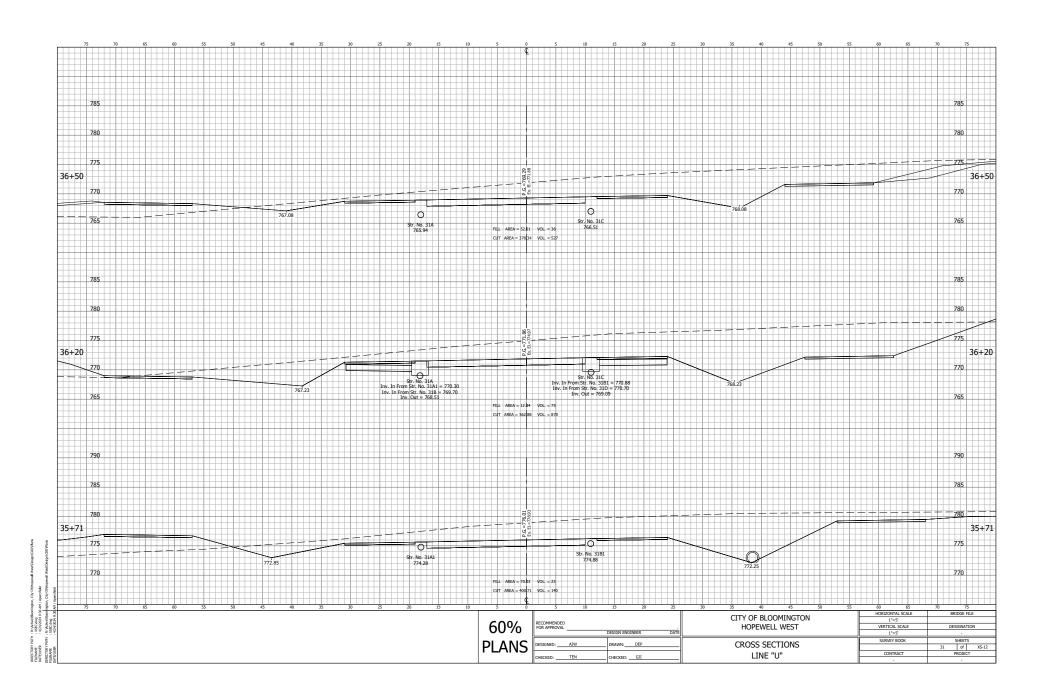


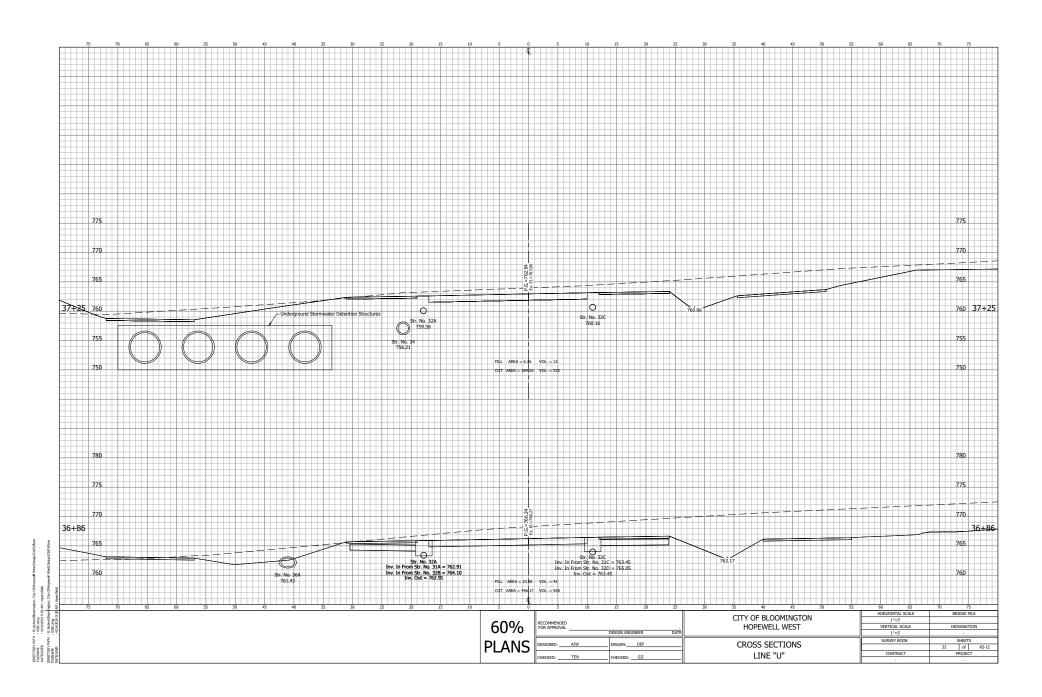


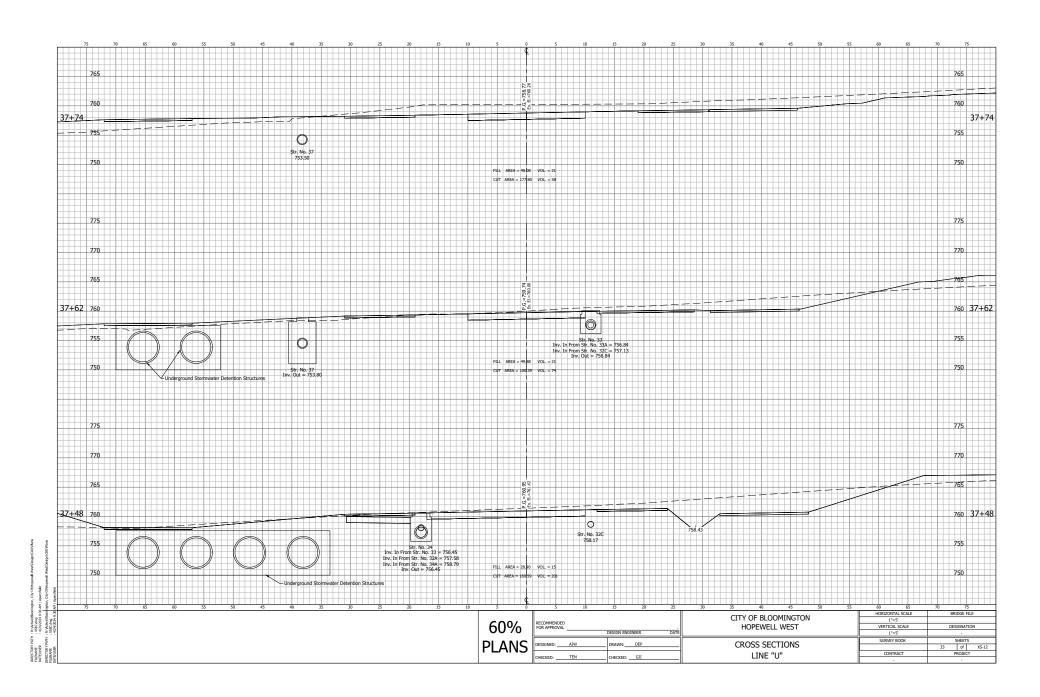


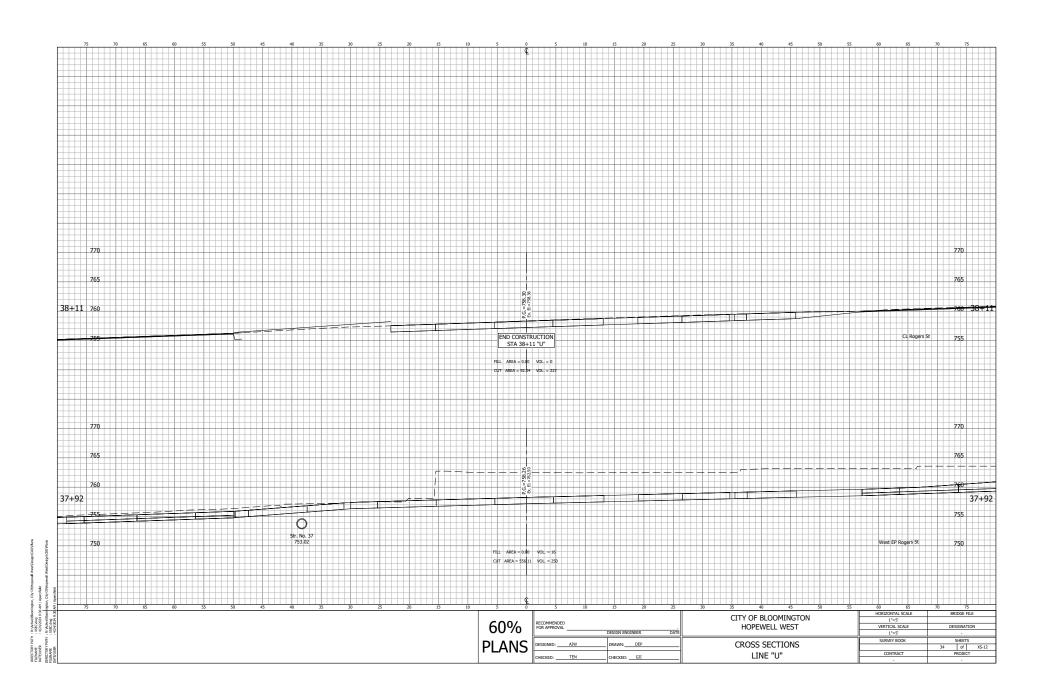


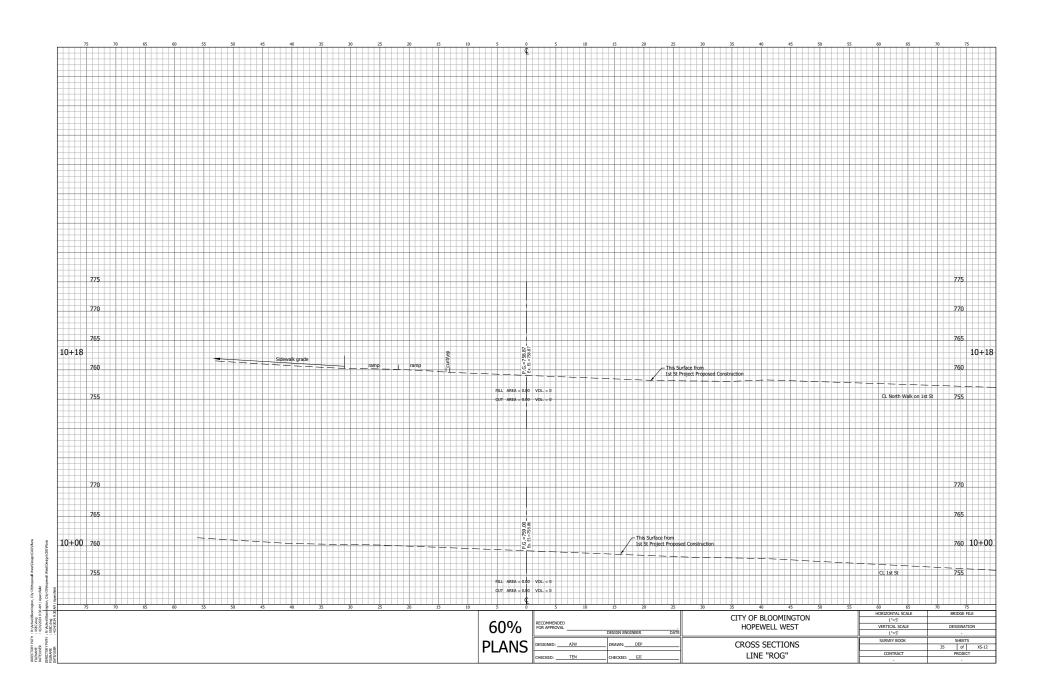


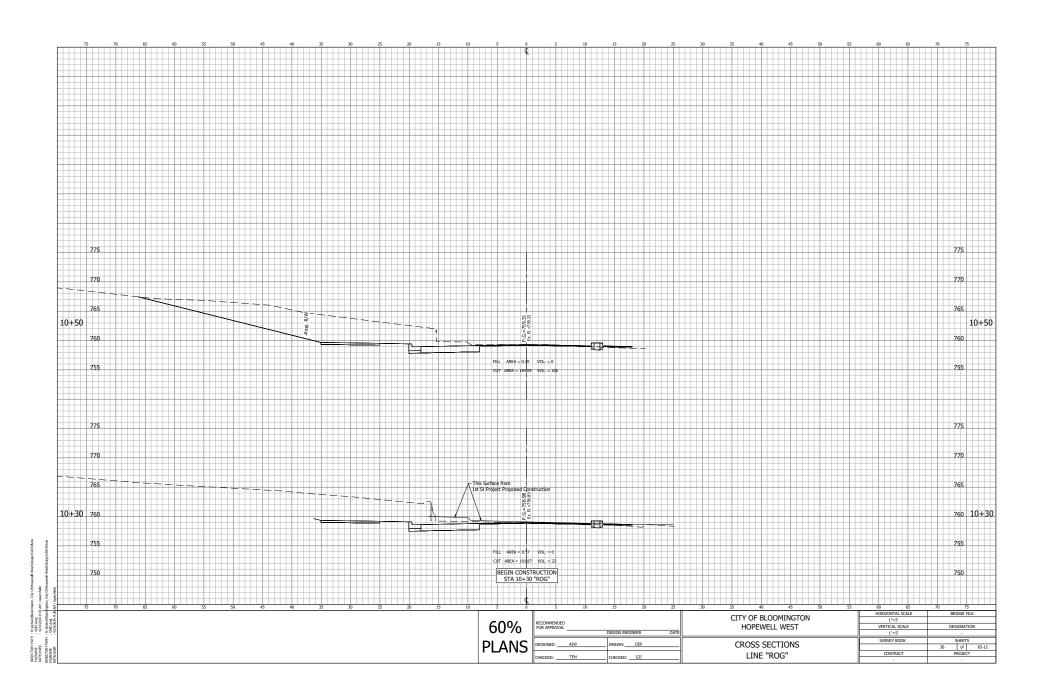


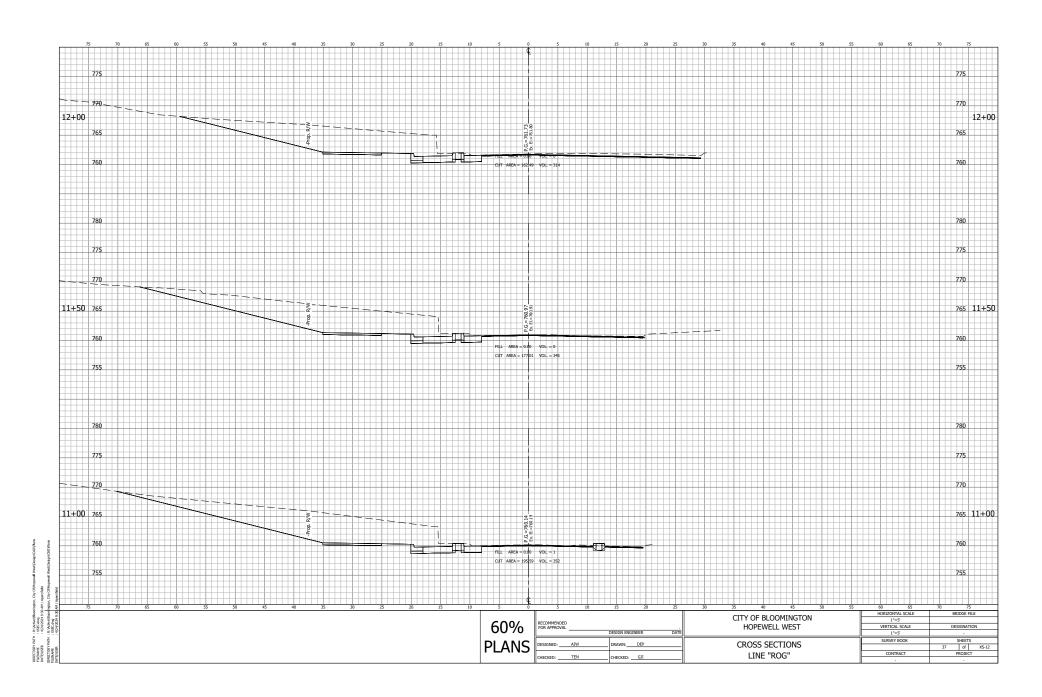


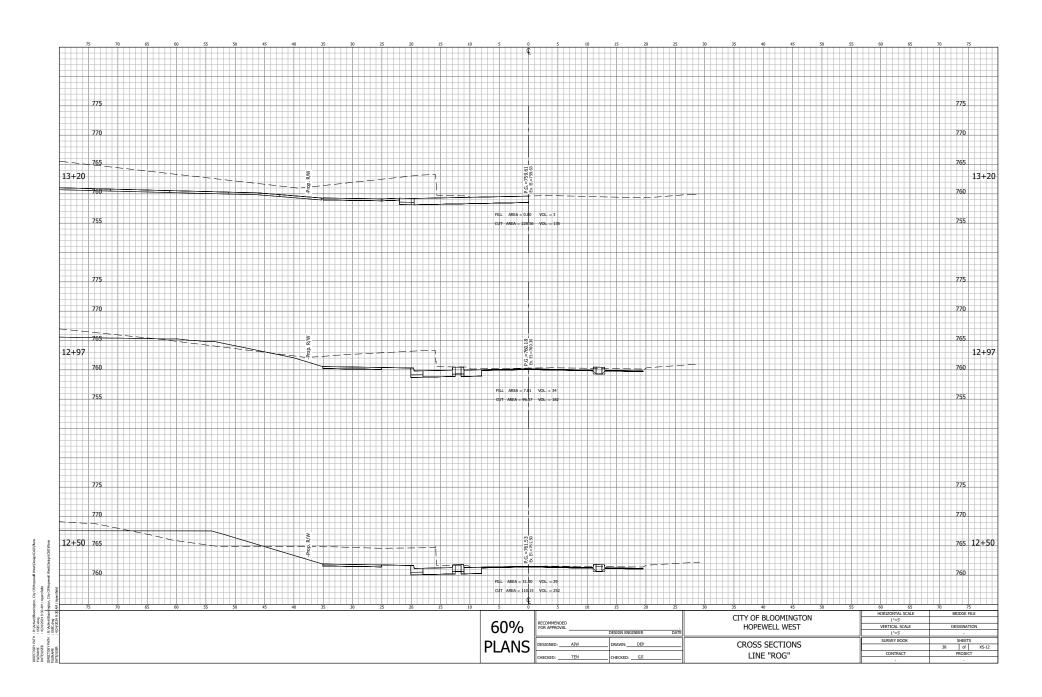


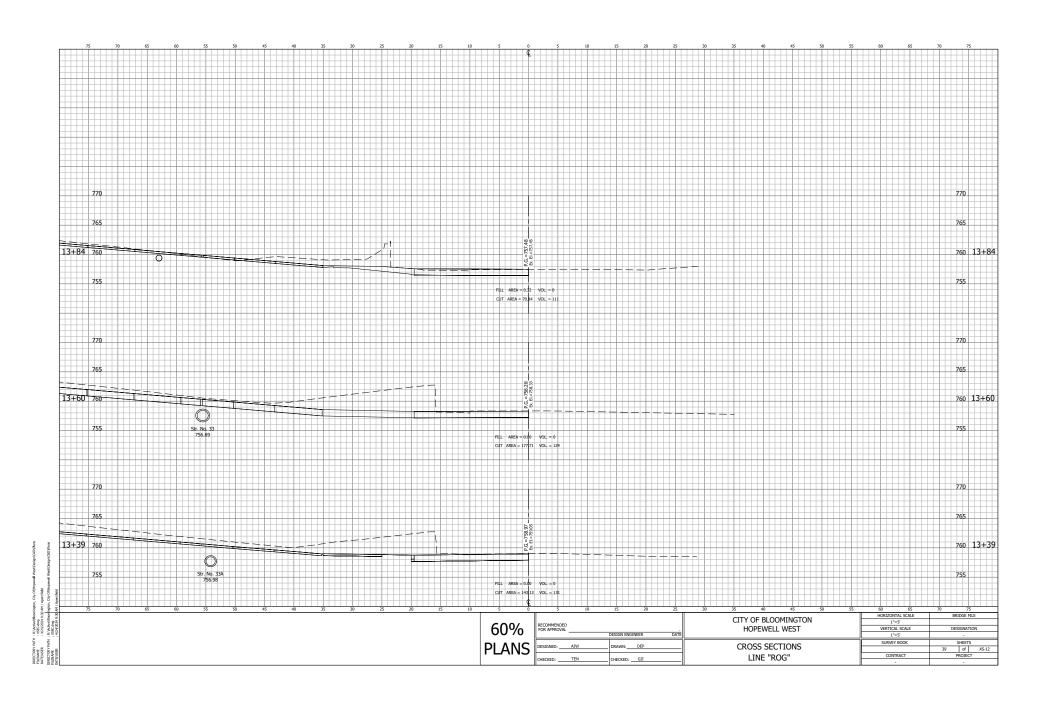


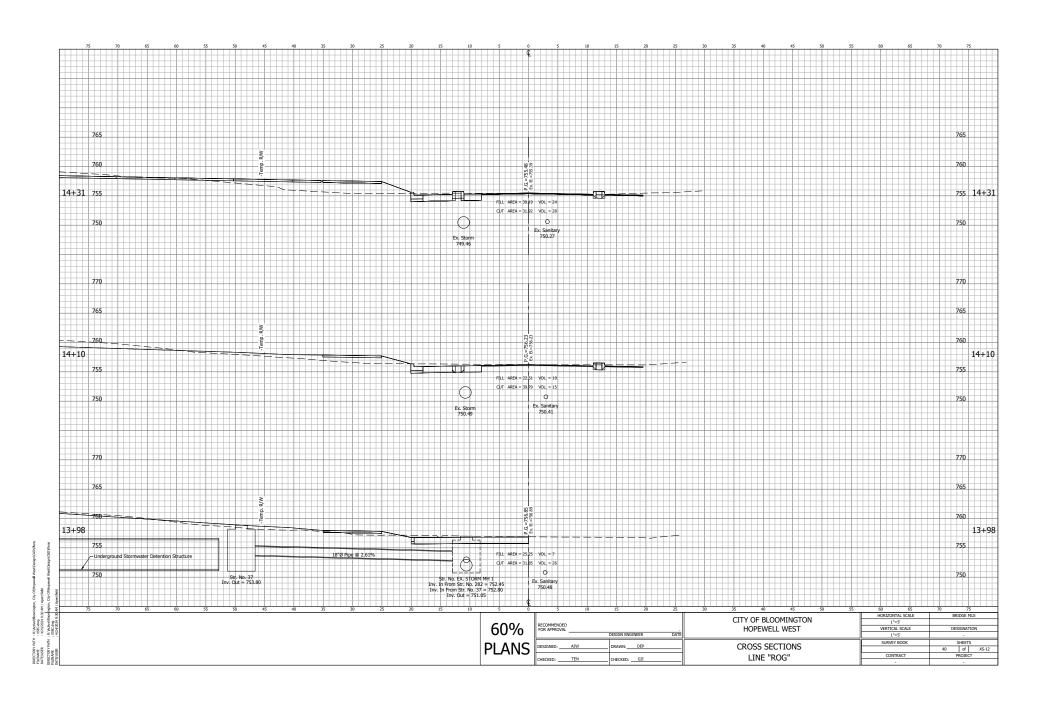


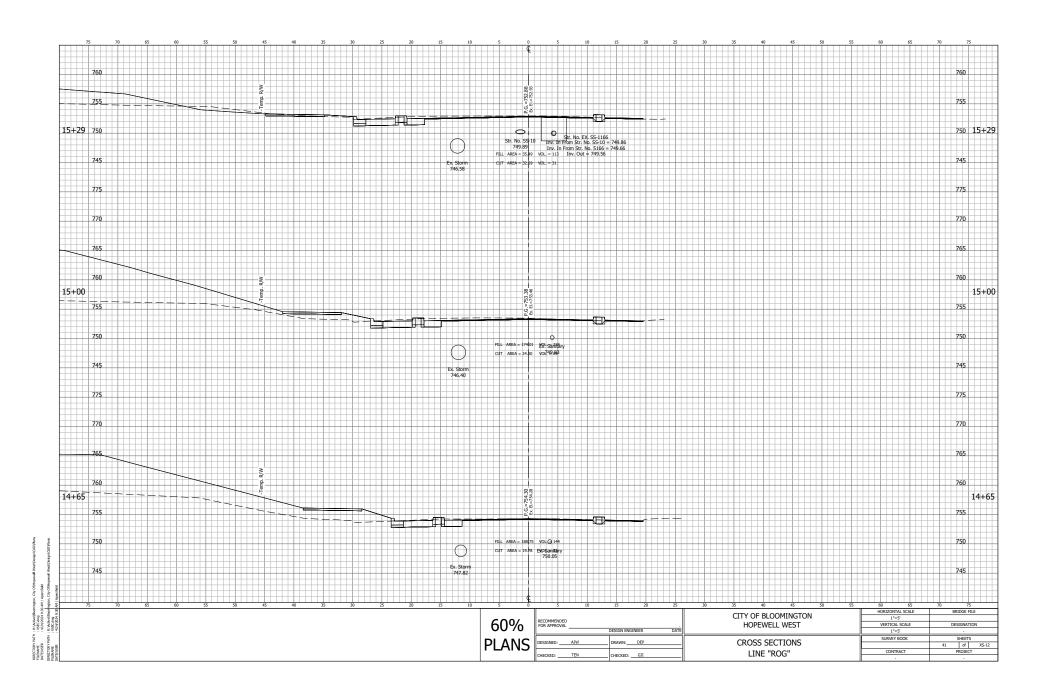


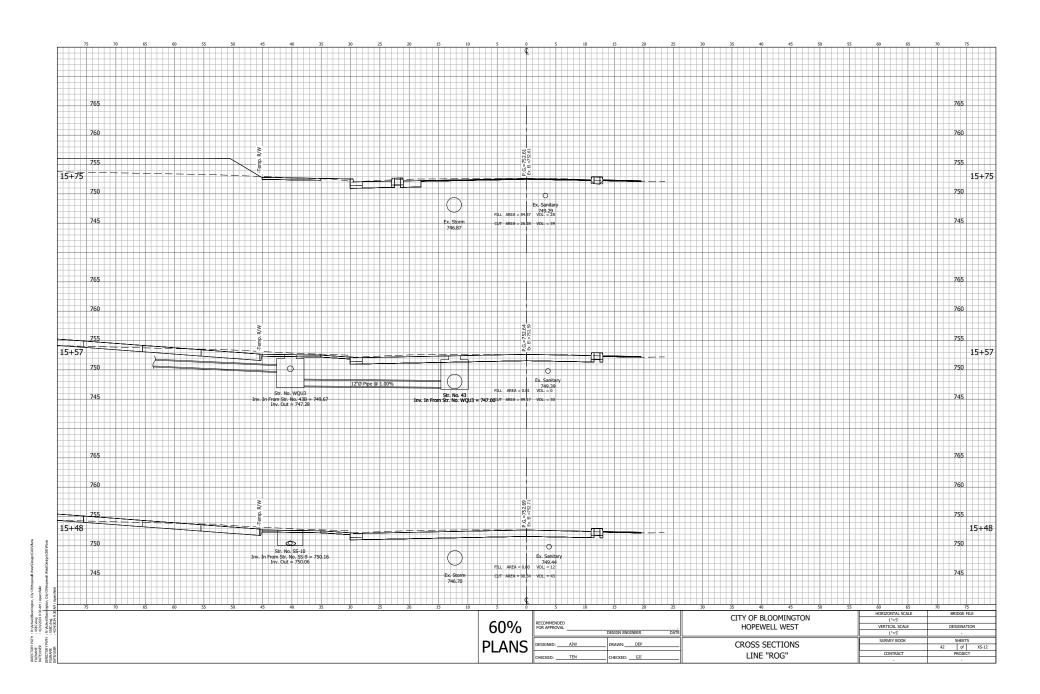


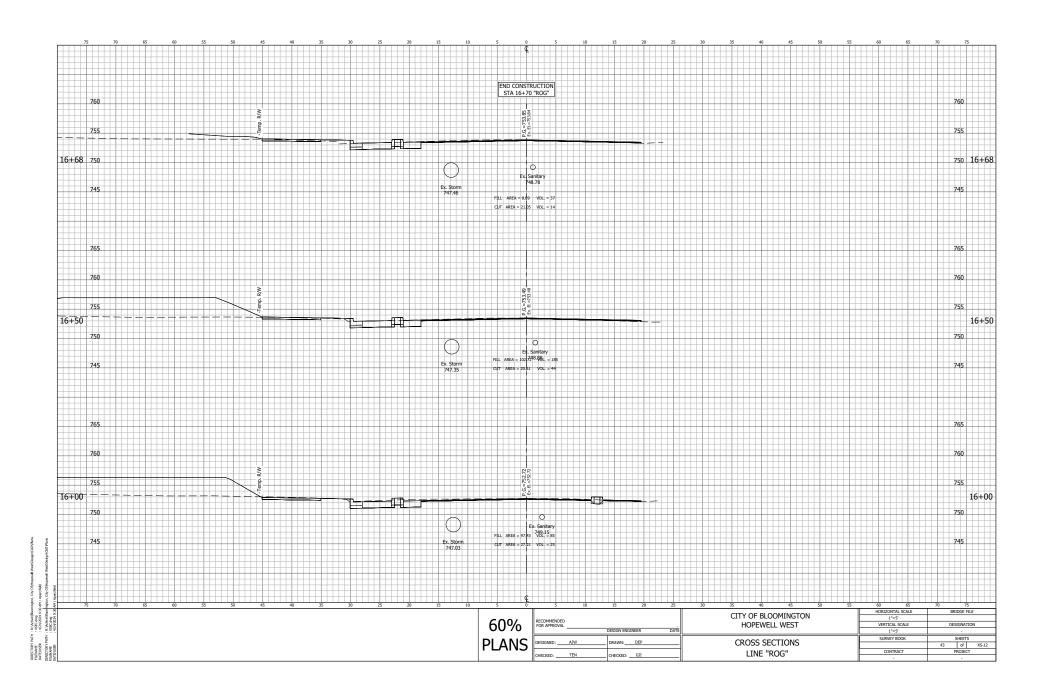


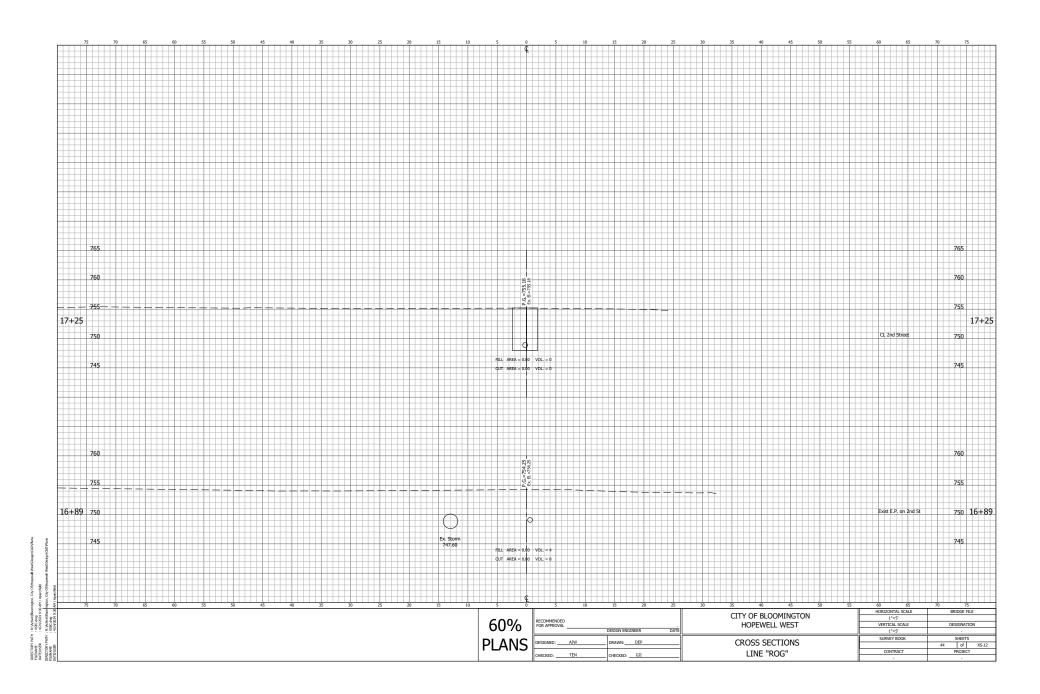


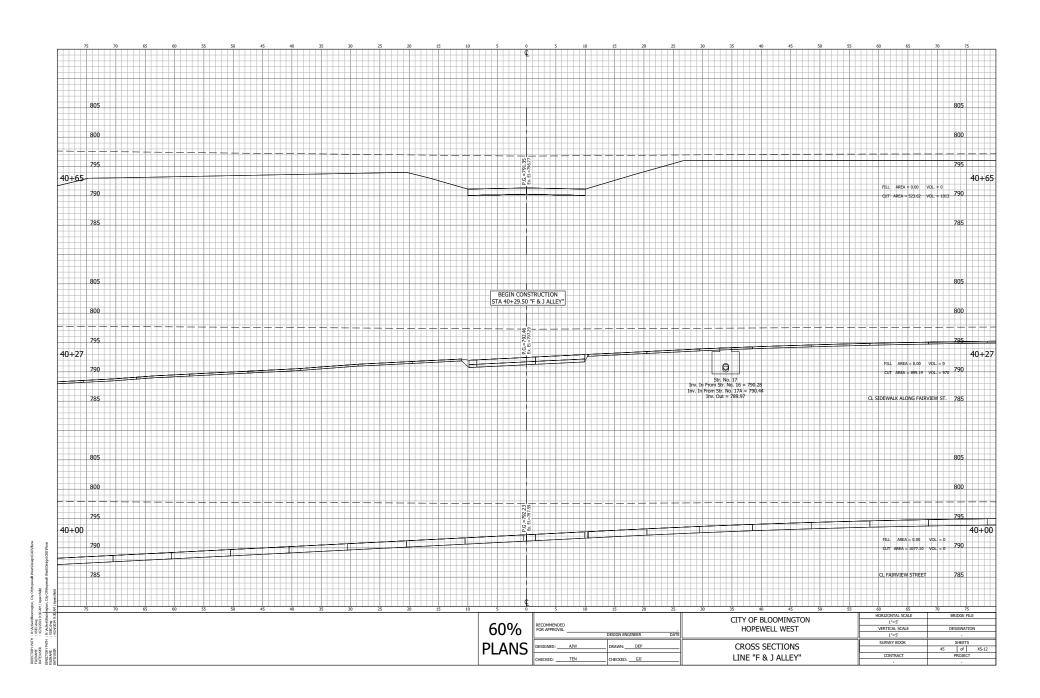


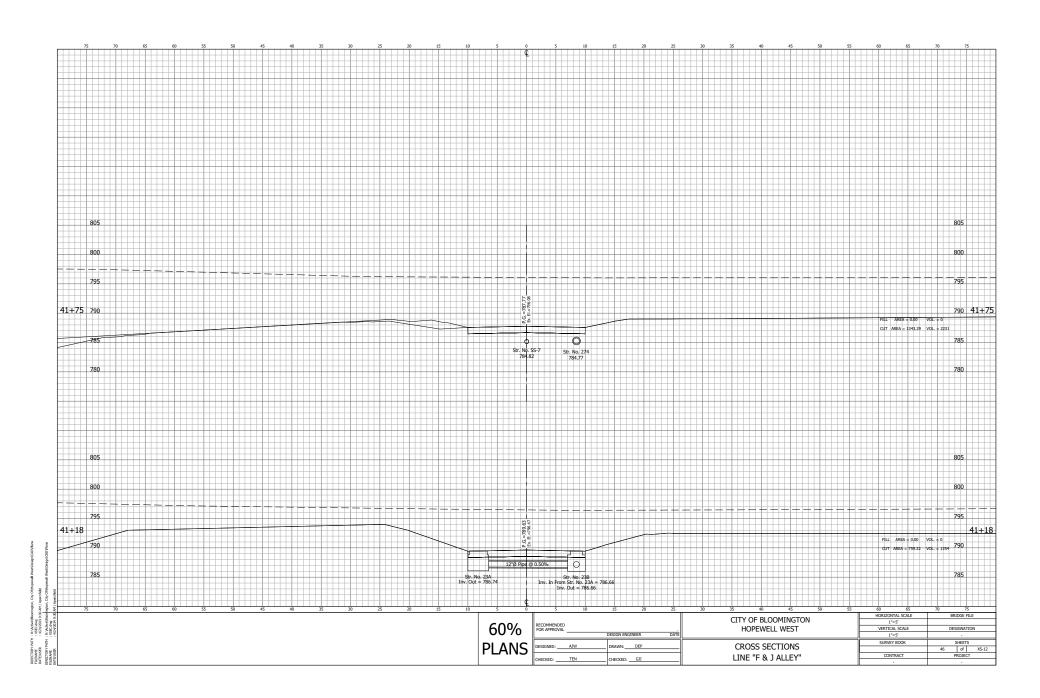


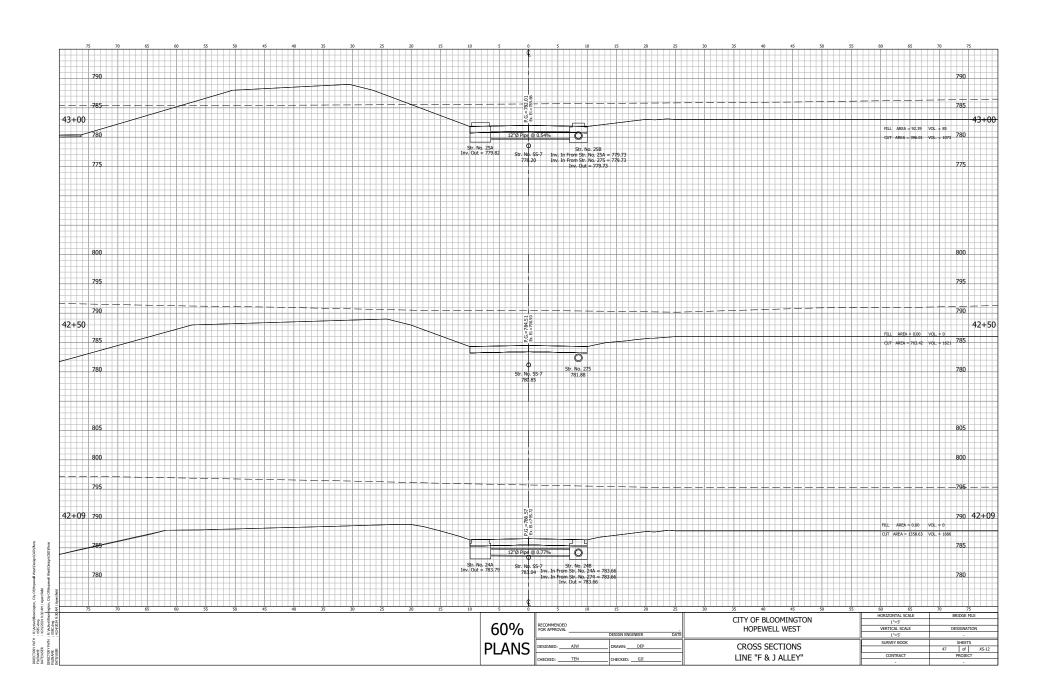


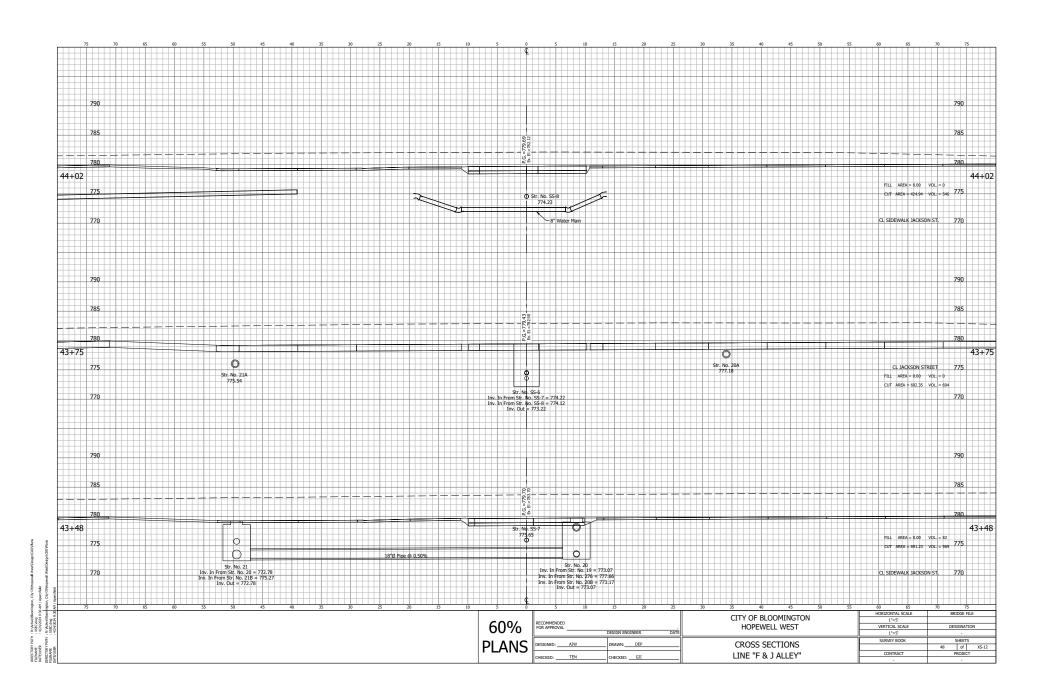


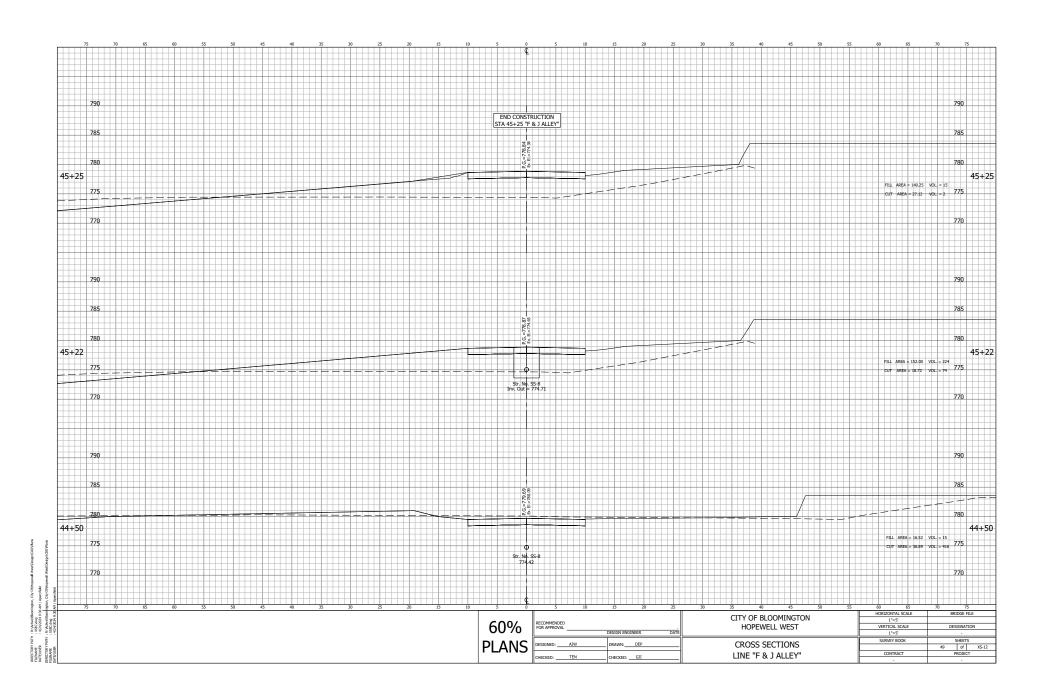


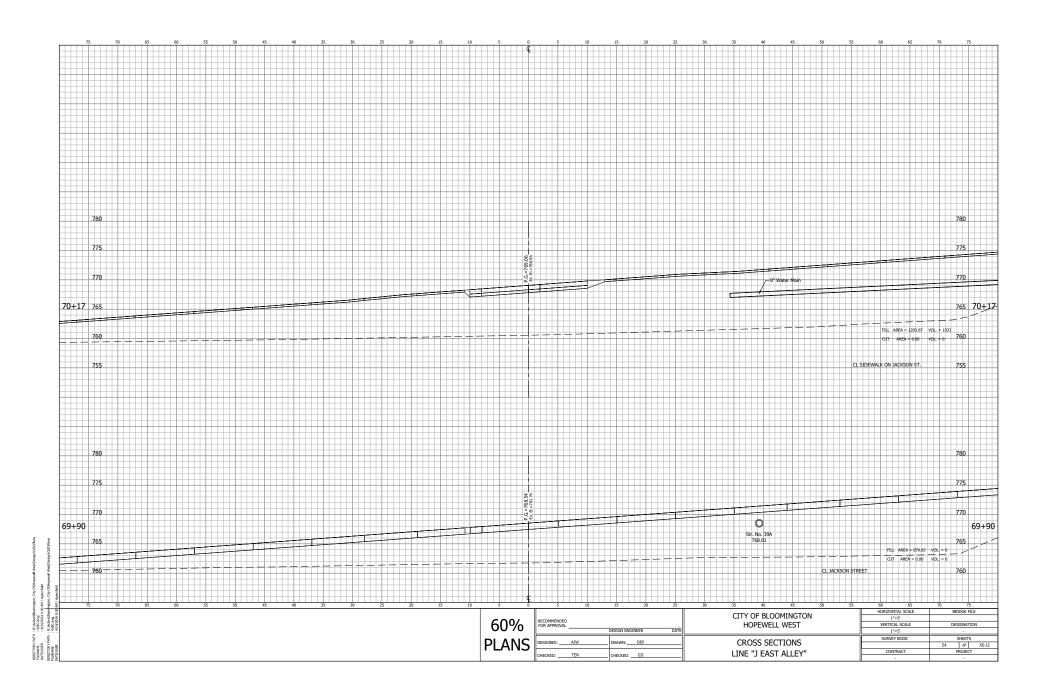


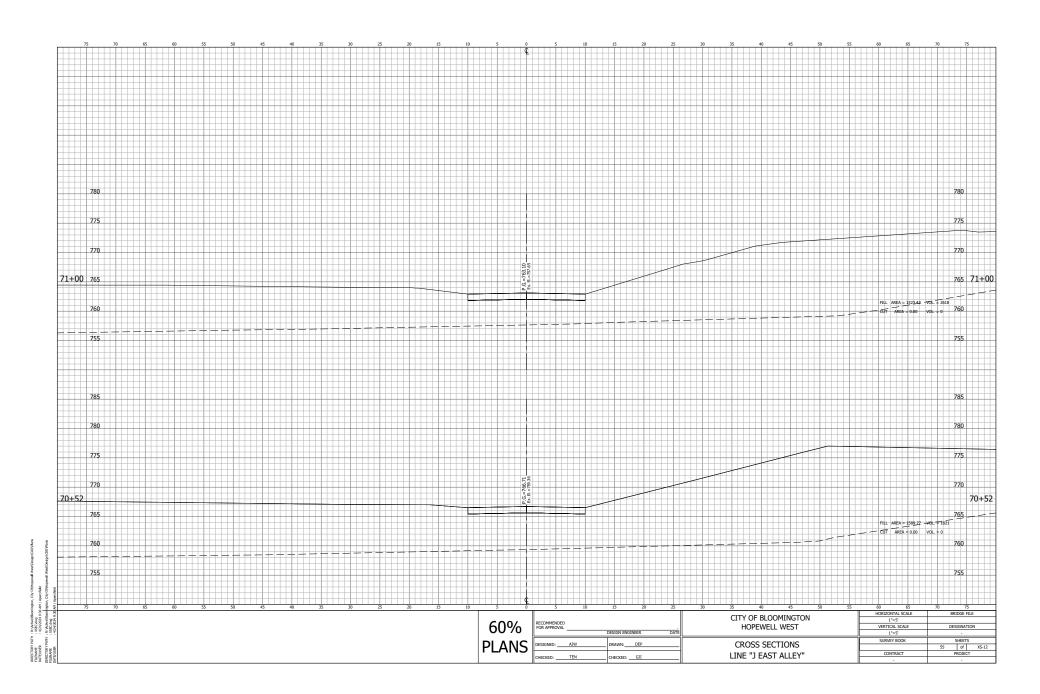


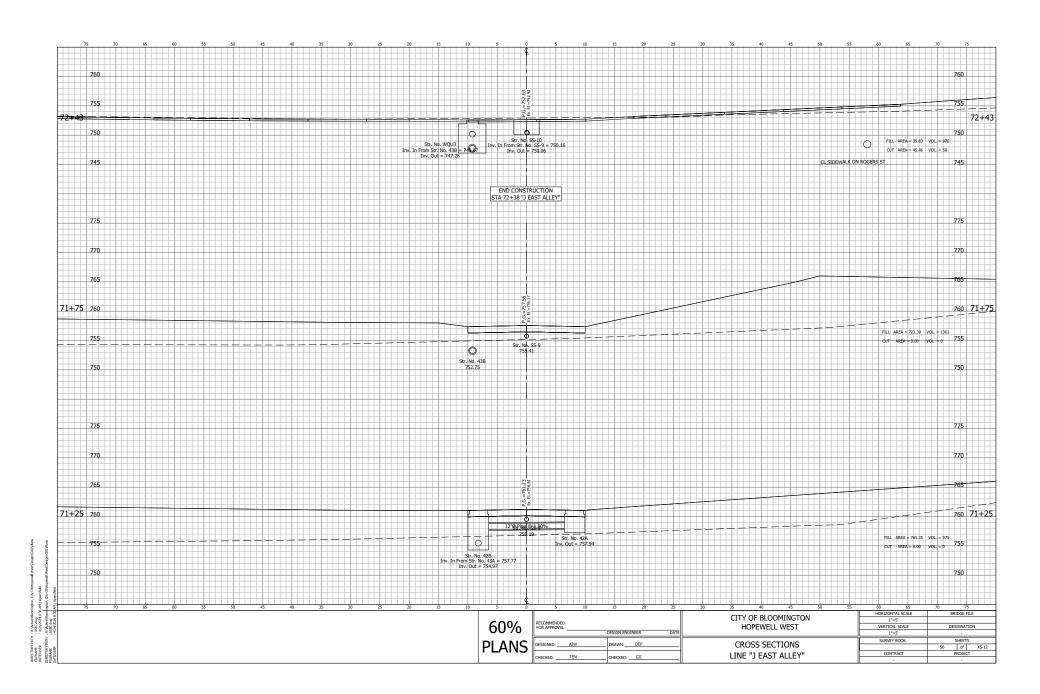


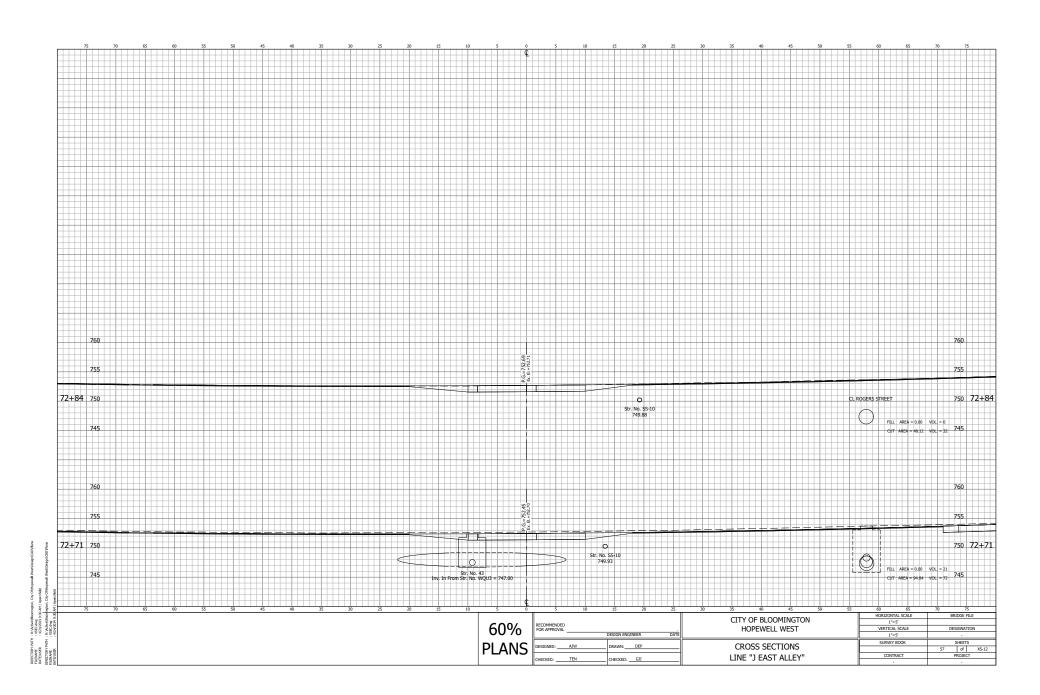










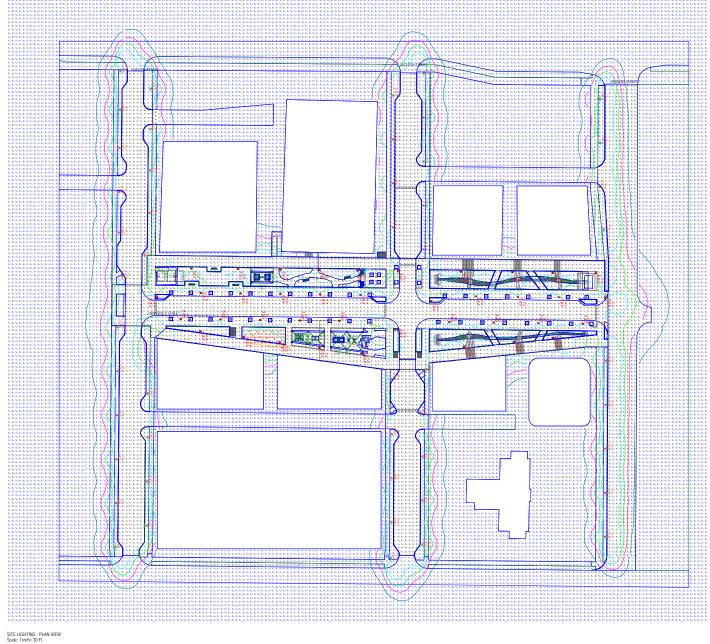


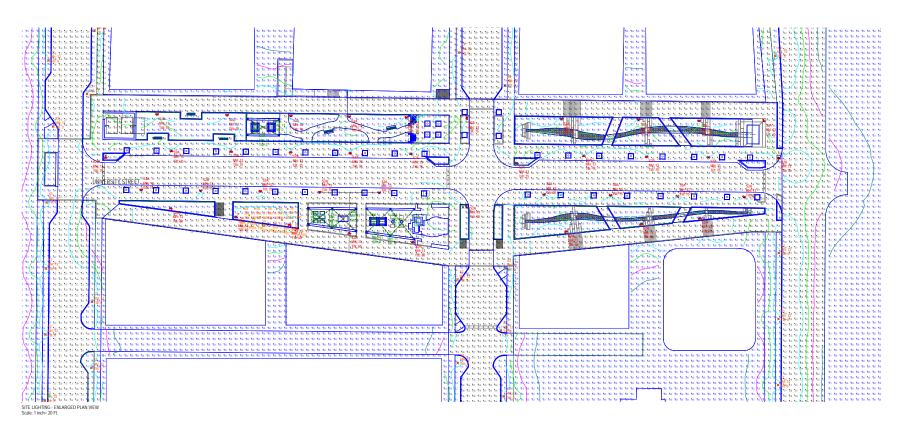
PROJECT NAME: BLOOMINGTON HOPEWELL SITE

PROJECT NUMBER: 20240024-SP DATE: 5-24-24

PHOTOMETRIC PLAN DESIGNED BY ESL-SPECTRUM WWW.ESL-SPECTRUM.COM PHONE: 317.951.2300







Liminak													
Project	2024002	4-SP BLO	DMINGTON HOR	EWELL -	SITE LIGHTIN	G R2 5-24-24							
Symbol	Oty	Label	Arrangement		Lum. Watts	Lum. Lumens		Manufacturer		ription			
_	39	S2A	2 @ 130 DEG		30	3496	0.850	LANDSCAPE		1882WF - 14	POLE		
- 0	102	S6A	Single		2.83	309	0.850	TEGAN		K-C-FG-xx			
0	11				13.6	1010	0.850	KIM LIGHTING		LTV83FF-NF-12L4K			
	49	3/3	Single		50.02	4623	0.850	DUKE	AVP	2 P103 XXXXX	30K R3 RNA		
6A FIXT	MOUNT URES A	RE SPACE	HTS ARE SPECI ED 3'-0" ON CEN	TER									
Calculat Project Label	MOUNT URES A ion Sum 2024002	ING HEIG RE SPACE TIARY 4-SP BLO	HTS ARE SPECI ED 3'-0" ON CEN	TER	SITE LIGHTIN	G R2 524-24 Units		Avq	Max	Mn	AvgMin	MaxMi	
Calculat Project Label	MOUNT URES A ion Sum 2024002	ING HEIG RE SPACE	HTS ARE SPECI ED 3'-0" ON CEN	TER PEWELL -	SITE LIGHTIN	G R2 5-24-24		Avg 128	Max 4.4	Min 0.2	Aug/Min 6.40	Max/Mi 22.00	
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SPECTRUM











RENDER IMAGE 4: NTS





HOPEWELL WEST SITE

CITY OF BLOOMINGTON, IN

FOR SECONDARY PLAT SUBMITTAL - APRIL 19, 2024

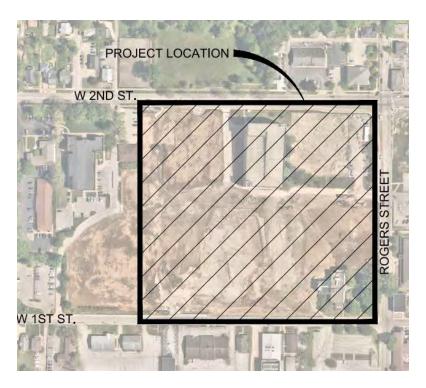
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HYDROLOGIC AND HYDRAULIC REPORT

INTRODUCTION

The City of Bloomington is planning to redevelop approximately 10 acres of the old IU Bloomington Hospital site into mixed-use areas and residential lots. Bloomington plans to reestablish the old "block" street grid system as part of this redevelopment and a new "greenway" street running east-west with landscape, streetscape, and multi-modal infrastructure features.

The site is bordered on the north by West 2nd Street, the east by South Rogers Street, and the south by West 1st Street. To the west, there are existing parcels owned by other entities that are remaining at this time.



FLOODZONE DESIGNATION

Based upon a scaled interpretation of the Flood Insurance Map No. 18105C0141D (Appendix pages A-1 through A-9) for Monroe County, Indiana, dated August 19, 2021, with data refreshed from October 2020, the subject tract is not located with Zone AE (Special Flood Hazard Area inundated by 100-year flood Base Flood Elevations determined) or Floodway Area Zone AE. The subject tract is located within Zone X (area of minimal risk between 1% and 0.2% annual chance floodplains. No base flood elevations or base flood depths are shown within the zone.

HYDROLOGIC ANALYSIS

The Rational Method is used as the basis of analysis for hydrologic design. Analysis for this project examined the 2-yr, 10-yr, and 100-year rainfall return period to determine the required predevelopment and post-development runoffs for evaluation.

The existing storm sewer shall be demolished in preparation for development and public infrastructure. The existing site is well drained and over 75% is hilly with slopes ranging from 6% to 12% with approximately 80% of the soil identified as clay and silt according to the Web Soil Survey.

All proposed storm sewer will tie into water quality and detention facilities as required. There are portions of the site, mostly along the northern half, that cannot topographically connect to the proposed detention facilities. The proposed system will outlet to existing storm networks along West 2nd Street to the north, and South Rogers Street to the east. These networks ultimately converge at South Rogers Street and continue east.

RAINFALL

The Intensity-Duration-Frequency (IDF) rainfall data used in the Rational Method analysis was taken from the City of Bloomington Utilities Department (CBU Standard Detail No. 16). The time of concentration for pre-developed site was calculated using the TR-55 methodology by adding the travel times for sheet flow, shallow concentrated flow, and channel flow. A minimum Time of Concentration of 5 minutes was used.

DETENTION REQUIREMENTS

Detention for the site is required. According to City of Bloomington Utility Construction Specifications, Detention shall be in accordance with the standards set forth in the latest issue of the Monroe County Storm Water Design Ordinance. As such, we followed the Monroe County Ordinance Section 761 to determine detention requirements and allowable release rates.

PRE-DEVELOPMENT CONDITIONS

Since this is a redevelopment site, we determined the detention requirements and allowable release rates based on the planned land use as outlined in the Master Plan for this site. The following tables indicate the applicable runoff coefficients and the areas associated with those surface types for pre-developed conditions. Since we are using Rational Method, we are instructed to use 0.2 according to ordinance. CBU requested a forested condition as a modification to this standard. Per Table AG-2 of Monroe County Ordinance 761, a steep woodland site results in a Runoff Coefficient, C of 0.50.

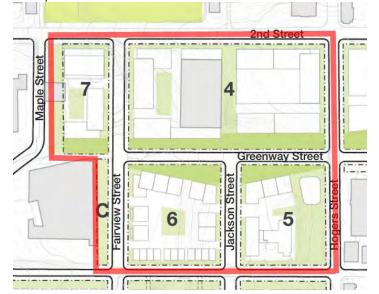
Pre-Development Runoff Coefficient					
Surface Type Runoff Coefficient, C Area, Ac.					
Steep (7%+) Woodland Condition 2-yr and 10-yr	0.50	10.83			
Steep (7%+) Woodland Condition 100-yr	0.50 * 1.25 = 0.625	10.83			

The existing drainage map and Time of Concentration path is shown in the Appendix (page A-14 & A-16 respectively). The resulting flows based on a 5-minute Time of Concentration are as follows:

Pre-Development Flows						
Return C i A Flow Rate, o						
2-Yr	0.5	5.48	10.83	29.67		
10-Yr	0.5	7.45	10.83	40.34		
100-Yr	0.625	10.42	10.83	70.53		

POST-DEVELOPMENT CONDITIONS

At this stage, ultimate development conditions have not been determined. As such, we are evaluating post-development conditions based on the ultimate plan as outlined in the Master Plan for the Hopewell West site. An exhibit of the site is shown below.



	Parcels		On-Par Greensp		1
	SF	AC	SF	AC	Γ
1	85,872	2.0	32,244	0.7	Γ
2	50,636	1.2	8,248	0.2	Γ
3	93,874	2.2	26,890	0.6	Γ
4	189,124	4.3	48,235	1.1	Γ
5	78,164	1.8	40,540	0.9	Γ
6	85,462	2.0	24,449	0.6	Γ
7	46,073	1.1	15,094	0.3	Г
8	88,168	2.0	20,358	0.5	Γ
9	94,211	2.2	22,639	0.5	Г
10	59,362	1.4	8,407	0.2	
Α	14,097	0.3	0.0	0.0	Γ
В	11,090	0.3	0.0	0.0	Ī
С	10,951	0.3	10,951	0.3	F
	907,083	(c-5)	258,056		+
Tota		21		6	

As part of the master plan, greenspace areas, and parcel areas were determined. As previously mentioned, the total runoff area, to the ultimate convergence point, is 10.83 acres. The maximum allowable impervious coverage area in this zoning district is 85%. The proposed runoff calculations are based on this maximum allowable coverage to be conservative. The remainder of area will be assumed to fall within public R/W and will be treated as impervious. The site will be graded to soften the slopes, but will still be rolling (2-7% slope). A breakdown and calculation for weighted coefficient for the entire site is tabulated below.

Post-Development Runoff Coefficients						
Surface Type Runoff Coefficient, C Ar						
Rolling (2-7%) Lawns 2-yr and 10-yr	0.21	10.83				
Rolling (2-7%) Lawns 100-yr	0.21 * 1.25 = 0.2625	10.83				
Asphalt 2-yr and 10-yr	0.82	10.83				
Asphalt 100-yr	0.82 * 1.25 = 1.025	10.83				

	Post-Development Runoff Coefficient (2-YR and 10-YR)							
Δre	a (sft)	Pervious Area (sft)	Pervious Coeff.	Impervious Area (sft)	Impervious Coeff.	Weighted Coeff.		
4	189124	28368.6	0.21	160755.4	0.82	0.73		
5	78164	11724.6	0.21	66439.4	0.82	0.73		
6	85462	12819.3	0.21	72642.7	0.82	0.73		
7	46073	6910.95	0.21	39162.05	0.82	0.73		
С	10951	10951	0.21	0	0.82	0.21		
ROW	61980	0	0.21	61980	0.82	0.82		
Total 471754						0.73		
	Post-Development Runoff Coefficient (100-YR)							

Area	a (sft)	Pervious Area (sft)	Pervious Coeff.	Impervious Area (sft)	Impervious Coeff.	Weighted Coeff.
4	189124	28368.6	0.2625	160755.4	1.025	0.91
5	78164	11724.6	0.2625	66439.4	1.025	0.91
6	85462	12819.3	0.2625	72642.7	1.025	0.91
7	46073	6910.95	0.2625	39162.05	1.025	0.91
С	10951	10951	0.2625	0	1.025	0.26
ROW	61980	0	0.2625	61980	1.025	1.03
Total	471754					0.91

The proposed drainage map and Time of Concentration path is shown in the Appendix (page A-15 & A-17 respectively). The resulting flows based on a 7.07-minute Time of Concentration are as follows:

Post-Development Flows						
Return Period	С	Flow Rate, cfs				
2-Yr	0.73	4.98	10.83	39.37		
10-Yr	0.73	6.74	10.83	53.29		
100-Yr	0.91	9.35	10.83	92.15		

ALLOWABLE DISCHARGE RATE & REQUIRED VOLUMES

Our Detention Storage Calculations utilize the rational method to calculate the necessary storage volume for the various return periods. The analysis scenarios for the 2-yr, 10-yr, and 100-yr can be found in the appendix (pages A-18 through A-20). Below is a summary of the peak results.

Required Storage Analysis				
Return Period Peak Storage, Acre-ft				
2-Yr	0.09			
10-Yr	0.12			
100-Yr	0.21			

VOLUME CALCULATION

The maximum required storage volume in acre-ft comes from the 100-year analysis. A volume of 0.21 acre-ft is required. It is proposed to employ an ADS MC-7200 Chamber system to detain the required 0.21 acre-ft runoff. The layout is shown in the plans along with size and volume characteristics meeting the required detention volume.

PROPOSED OUTLET STRUCTURE

The proposed outlet control structure is designed to release the post-development storm water below or matching the pre-development flow rates that were previously calculated. The calculations for the outlet control structure are provided in the appendix (pages A-21 through A-28). Below is a summary of the outlet control structure flow rates and the required storage compared to the values previously calculated as well as the proposed orifice diameters and the proposed storage elevation for each return period.

	Outlet Control Structure Analysis							
Return	Flow Ra	ate (cfs)		l Storage e-ft)	Orifice Diameter	Storage Elevation		
Period	Pre-Dev Post-Dev		Pre-Dev	Post-Dev	(ft)	Elevation		
2-Yr	29.67	28.97	.09	.10	2.1	754.04		
10-Yr	40.34	39.57	.12	.13	1.5	754.90		
100-Yr	70.53	70.53	.21	.21	Weir	757.54		

WATER QUALITY REQUIREMENTS

Water quality units have been provided at all outlet points. Water Quality units were designed by utilizing SCS Method Hydrographs for each of the outlet points. Below is a summary of the peak results and the proposed water quality units.

Required Water Quality Units Analysis					
Outlet Location	Peak Flow (cfs)	Proposed Structure	Proposed Str. Peak Flow (cfs)		
S. Fairview & W. 2 nd	1.94	AS-4	3.2		
S. Jackson & W. 2 nd	0.98	AS-2	1.1		
S. Jackson & S. Rogers	0.32	AS-2	1.1		
W University & S. Rogers	5.95	AS-6	6.3		

WATER OUALITY UNIT SIZING

There are four outlet points as part of this project. The first is a small basin on South Fairview that directly outlets to West 2nd Street. An Aquaswirl AS-4 unit shall be used to treat runoff prior to connecting to the existing infrastructure along West 2nd Street. The second is a small basin on South Jackson Street that directly outlets to 2nd Street. An Aquaswirl AS-2 unit shall be used to treat runoff prior to connecting to the existing infrastructure along West 2nd Street. The third is a small basin on South Jackson Street that directly outlets to South Rogers Street. An Aquaswirl AS-2 unit shall be used to treat runoff prior to connecting to the existing infrastructure along West 2nd Street. The fourth and final outlet structure is at the intersection of West University Street and South Rogers Street. An Aquaswirl AS-6 unit shall be used to treat runoff prior to connecting to the existing infrastructure along South Rogers Street. Aquaswirl Sizing Chart provided in the Appendix (page A-29).

STORM SEWER

The proposed drainage will utilize curb inlets, roadside ditches, and new storm sewer to replicate the distribution of storm water in the existing condition. Linear trench drains will be utilized to collect runoff in areas where curbs aren't present and shared use spaces are prevalent.

The City of Bloomington Utility Standards and Design Guide, INDOT Design Manual (IDM 2013), and Monroe County Storm Water Standards and Ordinance were used to design the system. Applicable sections of the Manual are attached in the Appendx (pages A-10 through A-13) for reference and values used are highlighted. More specifics are discussed in the Hydrologic and Hydraulic Analysis sections of this report.

The proposed system was analyzed using the Rational Method. The weighted runoff coefficients were calculated using the Table AG-1 of Monroe County Ordinance 761. A coefficient for impervious areas (asphalt or concrete pavement) of 0.82 and a coefficient for pervious areas (Lawn, Clay,Rolling) of 0.21 were used for 10 year Storm analysis. A coefficient for impervious areas (asphalt or concrete pavement) of 1.03 and a coefficient for pervious areas (Lawn, Clay,Rolling) of 0.26 were used for 100 year Storm analysis. Drainage area tables summarizing drainage areas for each basin are provided in the Appendix (pages A-30 through A-32).

Where practical, a conservative approach was used, and the basins were evaluated using the 5-minute minimum time of concentration. The rainfall intensities and depths for various return periods and storm durations are provided by the City of Bloomington Utilities Department (CBU Standard Detail No. 16). Applicable Manning's n values used for sheet flow and channel flow calculations are located in the Appendix (page A-13). The proposed Time of Concentration Worksheets are presented in Appendix (pages A-33 and A-34).

PROPOSED SYSTEM HYDRAULIC ANALYSIS

The development's proposed structures were analyzed for a design frequency of 10% Annual (10-year storm) for gravity flow. Since this site is extremely sloped, there are no sags present and were not evaluated.

Inlets are spaced to meet the City of Bloomington's requirements for local roads by providing a minimum of one 8 ft lane for two-lane facilities. Per Table AG-4 of Monroe County Ordinance 761 a Manning's n of 0.013 was utilized for the concrete gutters proposed throughout the development. The inlet spacing and spread worksheet for the proposed structures is presented in the Appendix (page A-35).

The proposed system will utilize reinforced concrete pipe (RCP). A Manning's "n" for concrete pipe of 0.013 was used. The proposed system's pipe capacities are summarized in the Proposed Pipe Hydraulics table located in the Appendix (A-36). The pipe capacity table shows the Q10 Efficiency for the proposed system in Column 17.

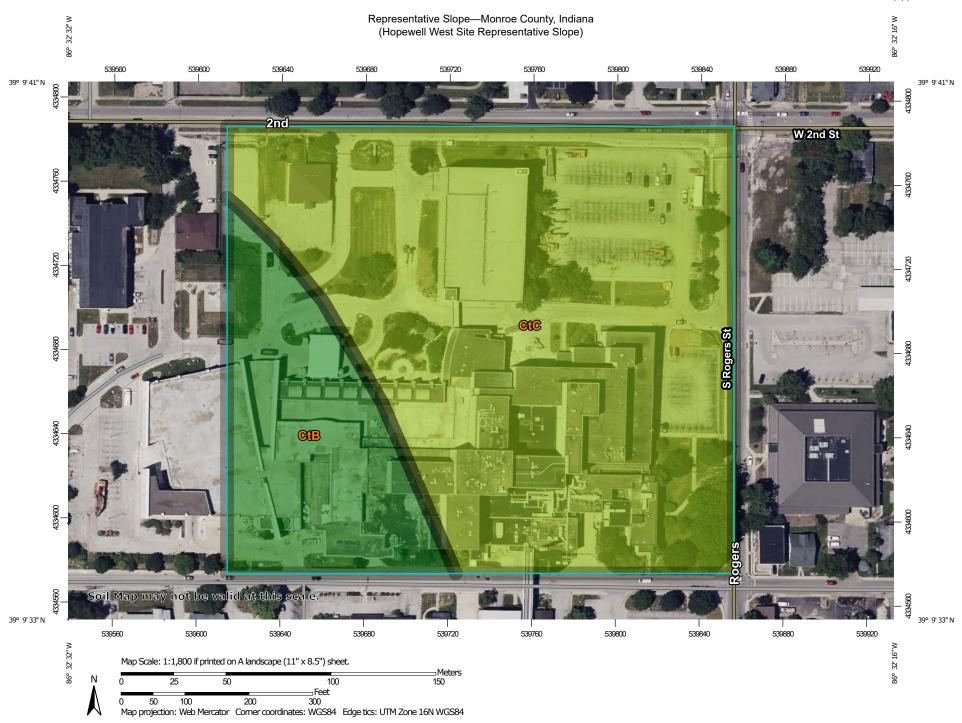
SCOUR PROTECTION

A minimum full-flow velocity of 2.5 ft/s is desirable to prevent sedimentation from occurring in the pipes. The recommended maximum storm-sewer velocity is 10.0 ft/s. Per Indiana Design Manual Figure 203-2D (appendix page A-37), revetment riprap can be placed at the ends of the proposed pipes based on the outlet velocity being less than or equal to 6.5 ft/s. All outlet velocities at proposed pipes are less than 6.5 ft/s, so revetment riprap will be utilized.

PREPARER CONTACT INFORMATION

Andrew J. Wolf, PE CrossRoad Engineers, PC 115 N. 17th Avenue Beech Grove, IN 46107 (317) 780-1555 x124 awolf@crossroadengineers.com

APPENDIX



MAP LEGEND

Area of Interest (AOI) Transportation Area of Interest (AOI) Rails Soils Interstate Highways **Soil Rating Polygons** US Routes 0 - 5 Major Roads 5 - 15 Local Roads 15 - 45 Background 45 - 60 Aerial Photography 60 - 100 Not rated or not available Soil Rating Lines 0 - 5 5 - 15 15 - 45 45 - 60 60 - 100 Not rated or not available **Soil Rating Points** 0 - 5 5 - 15 15 - 45 45 - 60 60 - 100 Not rated or not available **Water Features** Streams and Canals

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15.800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Web Soil Survey URL.

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Monroe County, Indiana Survey Area Data: Version 30, Sep 1, 2023

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 15, 2022—Jun 21, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Representative Slope

Map unit symbol	Map unit name	Rating (percent)	Acres in AOI	Percent of AOI
CtB	Crider-Urban land complex, 2 to 6 percent slopes	4.1	2.9	22.4%
CtC	Crider-Urban land complex, 6 to 12 percent slopes	9.0	9.9	77.6%
Totals for Area of Interest			12.8	100.0%

Description

Slope gradient is the difference in elevation between two points, expressed as a percentage of the distance between those points.

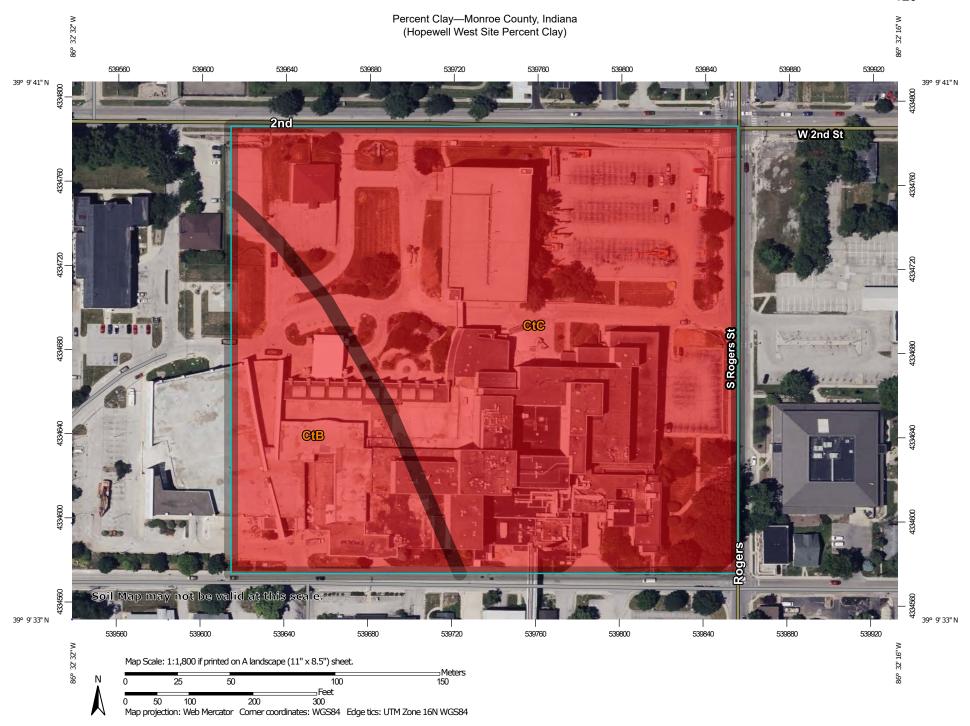
The slope gradient is actually recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.

Rating Options

Units of Measure: percent

Aggregation Method: Dominant Component Component Percent Cutoff: None Specified

Tie-break Rule: Higher Interpret Nulls as Zero: No



MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Rating Polygons



= 39.5



Not rated or not available

Soil Rating Lines



= 39.5



Not rated or not available

Soil Rating Points



= 39.5

Rails

Not rated or not available

Water Features



Streams and Canals

Transportation



Interstate Highways



US Routes



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15.800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

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Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Jun 15, 2022—Jun 21, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Percent Clay

Map unit symbol	Map unit name	Rating (percent)	Acres in AOI	Percent of AOI		
CtB	Crider-Urban land complex, 2 to 6 percent slopes	39.5	2.9	22.4%		
CtC	Crider-Urban land complex, 6 to 12 percent slopes	39.5	9.9	77.6%		
Totals for Area of Interest			12.8	100.0%		

Description

Clay as a soil separate consists of mineral soil particles that are less than 0.002 millimeter in diameter. The estimated clay content of each soil layer is given as a percentage, by weight, of the soil material that is less than 2 millimeters in diameter. The amount and kind of clay affect the fertility and physical condition of the soil and the ability of the soil to adsorb cations and to retain moisture. They influence shrink-swell potential, saturated hydraulic conductivity (Ksat), plasticity, the ease of soil dispersion, and other soil properties. The amount and kind of clay in a soil also affect tillage and earth-moving operations.

Most of the material is in one of three groups of clay minerals or a mixture of these clay minerals. The groups are kaolinite, smectite, and hydrous mica, the best known member of which is illite.

For each soil layer, this attribute is actually recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.

Rating Options

Units of Measure: percent

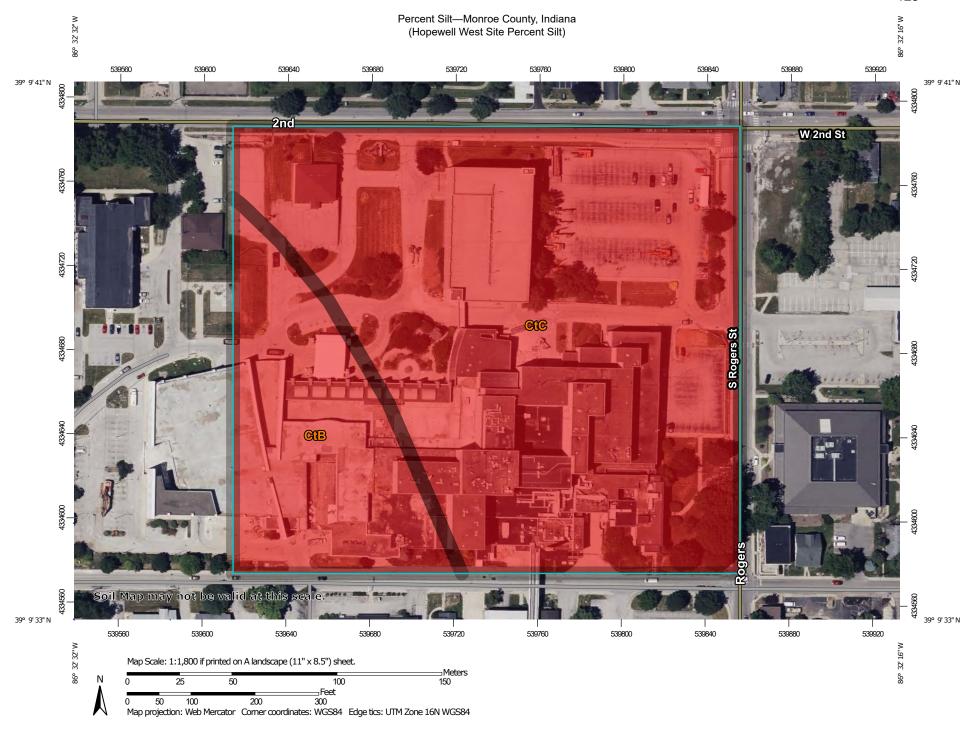
Aggregation Method: Dominant Component Component Percent Cutoff: None Specified

Tie-break Rule: Higher Interpret Nulls as Zero: No

Layer Options (Horizon Aggregation Method): Depth Range (Weighted Average)

Top Depth: 12
Bottom Depth: 120

Units of Measure: Centimeters



MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Rating Polygons



= 41.3



Not rated or not available

Soil Rating Lines



= 41.3



Not rated or not available

Soil Rating Points



= 41.3

Not rated or not available

Water Features



Streams and Canals

Transportation





Interstate Highways



US Routes

Rails



Major Roads



Local Roads

Background



Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15.800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Monroe County, Indiana Survey Area Data: Version 30, Sep 1, 2023

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Jun 15, 2022—Jun 21, 2022

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Percent Silt

Map unit symbol	Map unit name	Rating (percent)	Acres in AOI	Percent of AOI
CtB	Crider-Urban land complex, 2 to 6 percent slopes	41.3	2.9	22.4%
CtC	Crider-Urban land complex, 6 to 12 percent slopes	41.3	9.9	77.6%
Totals for Area of Interest			12.8	100.0%

Description

Silt as a soil separate consists of mineral soil particles that are 0.002 to 0.05 millimeter in diameter. In the database, the estimated silt content of each soil layer is given as a percentage, by weight, of the soil material that is less than 2 millimeters in diameter.

The content of sand, silt, and clay affects the physical behavior of a soil. Particle size is important for engineering and agronomic interpretations, for determination of soil hydrologic qualities, and for soil classification

For each soil layer, this attribute is actually recorded as three separate values in the database. A low value and a high value indicate the range of this attribute for the soil component. A "representative" value indicates the expected value of this attribute for the component. For this soil property, only the representative value is used.

Rating Options

Units of Measure: percent

Aggregation Method: Dominant Component Component Percent Cutoff: None Specified

Tie-break Rule: Higher Interpret Nulls as Zero: No

Layer Options (Horizon Aggregation Method): Depth Range (Weighted Average)

Top Depth: 12
Bottom Depth: 120

Units of Measure: Inches

TABLE AG-2

Rural Runoff Coefficients

Type of Surface	Runoff Coefficient	
Woodland (Clay)		
Flat	0.30	
Rolling	0.35	
Steep Steep	0.50	
Pasture (Clay)		
Flat	0.30	
Rolling	0.36	
Steep	0.42	
Cloop	VI-12	

The coefficients of this tabulation are applicable to storms of five to ten year frequencies. Coefficients for less frequent higher intensity storms shall be modified as follows:

Return Period (yrs)	Multiply "C" by
25	1.1
50	1.2
100	1.25

TABLE AG-1

Urban Runoff Coefficients

Type of Surface	Runoff Coefficient
Asphalt	0.82
Concrete	0.85
Roof	0.85
Lawns (Clay)	
Flat (0-2% Slope)	0.16
Rolling (2-7% Slope)	0.21
Steep (Greater than 7%)	0.30

The coefficients of this tabulation are applicable to storms of five to ten year frequencies. Coefficients for less frequent higher intensity storms shall be modified as follows:

Return Period (yrs)	Multiply "C" by
25	1.1
50	1.2
100	1.25

Updated Rainfall Data

Source: http://hdsc.nws.noaa.gov/hdsc/pfds/index.html

Rainfall Duration

Durc	ation	Return Period Rainfall Depth (in)						
Hours	Minutes	1 year	2 year	5 year	10 year	25 year	50 year	100 year
0.0833	5	0.38	0.46	0.55	0.62	0.72	0.79	0.87
0.1667	10	0.60	0.71	0.85	0.96	1.10	1.20	1.31
0.25	15	0.73	0.87	1.05	1.18	1.35	1.49	1.62
0.5	30	0.97	1.17	1.44	1.64	1.91	2.13	2.34
1	60	1.18	1.43	1.80	2.08	2.48	2.80	3.13
2	120	1.38	1.67	2.11	2.46	2.97	3.38	3.83
3	180	1.47	1.79	2.26	2.65	3.21	3.67	4.18
6	360	1. <i>7</i> 8	2.15	2.73	3.20	3.89	4.47	5.11
12	720	2.11	2.54	3.18	3.70	4.43	5.05	5.70
24	1440	2.55	3.07	3.82	4.44	5.31	6.04	6.80

Rainfall Intensity

Durc	ation	Return Period Rainfall Intensity (in/hr)						
Hours	Minutes	1 year	2 year	5 year	10 year	25 year	50 year	100 year
0.0833	5	4.61	5.48	6.59	7.45	8.60	9.52	10.42
0.1667	10	3.58	4.28	5.12	5.75	6.58	7.21	7.84
0.25	15	2.92	3.49	4.19	4.72	5.42	5.95	6.49
0.5	30	1.93	2.34	2.87	3.28	3.83	4.25	4.69
1	60	1.18	1.43	1.80	2.08	2.48	2.80	3.13
2	120	0.69	0.84	1.05	1.23	1.48	1.69	1.91
3	180	0.49	0.59	0.75	0.88	1.07	1.22	1.39
6	360	0.30	0.36	0.46	0.53	0.65	0.75	0.85
12	720	0.18	0.21	0.26	0.31	0.37	0.42	0.47
24	1440	0.11	0.13	0.16	0.18	0.22	0.25	0.28

REVISED 10/13/2011 G.N.

City of Bloomington Utilities Engineering Department

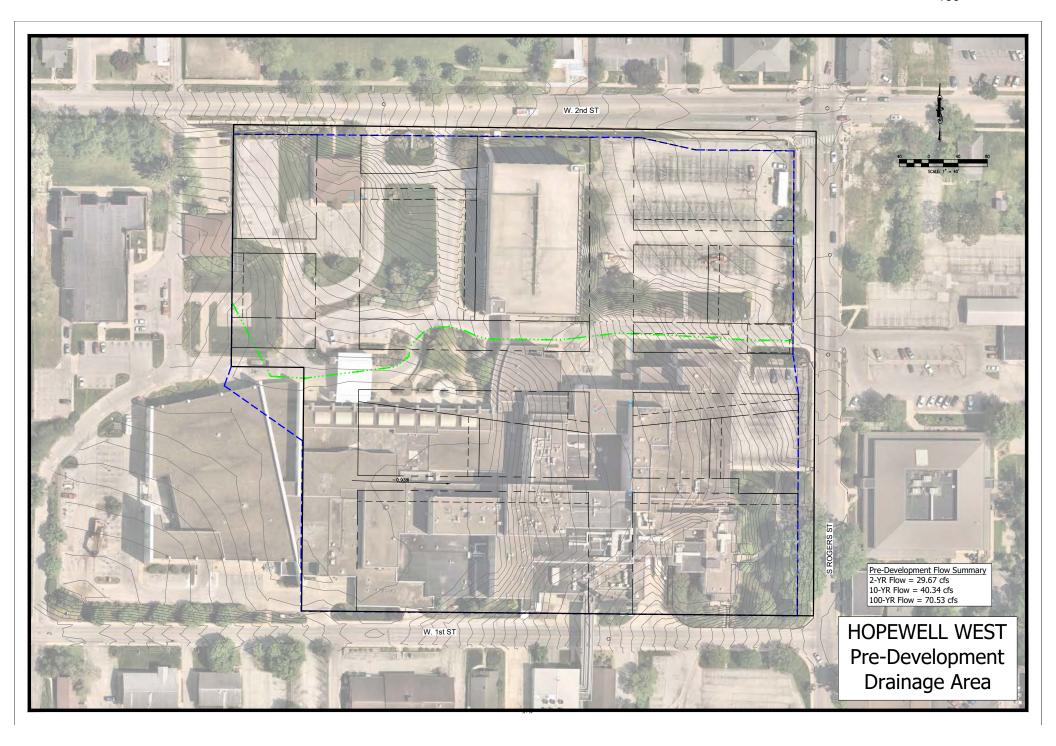
NO SCALE DRAWING FILE:
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M. Hicks I:\COMMON\STA I:\COMMON\STANDARD DRAWINGS\STD16.DWG

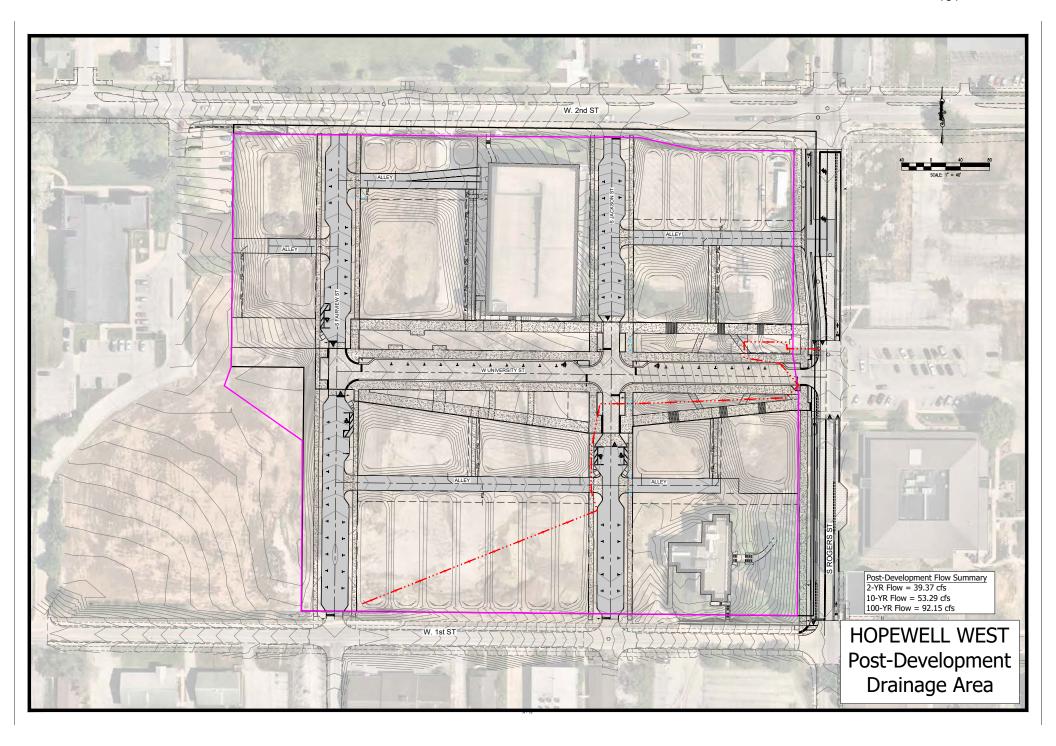
DEPTH AND INTENSITY DURATION FREQUENCY TABLES

STANDARD DETAIL NUMBER

TABLE AG-4
Typical Values of Manning's n

Material	Manning's n	Desirable Maximum Velocities
Closed Conduits		
Concrete	0.013	15.f.p.s.
Vitrified Clay	0.013	15 f.p.s.
Brick	0.015	15 f.p.s.
Cast Iron	0.013	15 f.p.s.
Circular Corrugated Me	tal Pipe, Annular Corrugatio	ns, 2 2/3 x 2 in.
Unpaved	0.024	7 f.p.s.
25% Paved	0.021	7 f.p.s.
50% Paved	0.018	7 f.p.s.
100% Paved	0.013	7 f.p.s.
Circular Metal Pipe, Hel	ical, 2 2/3 x 2 in. Unpaved Co	orrugations
12"	0.011	_
18"	0.013	
24"	0.015	
36"	0.018	
48"	0.020	
60" or larger	0.021	
Corrugated Polyethylene	0.012	15 f.p.s.
Smooth Interior Pipe		
Concrete Culverts	0.013	
Open Channels		
Concrete Trowel Finish	0.013	
Concrete, Broom or Float		
Finish	0.015	
Gunite	0.018	
Riprap Placed	0.030	
Riprap Dumped	0.035	
Gabion	0.028	
New Earth (Uniform, Sodded,		
Clay)	0.025	
Existing Earth (Fairly Uniform With		
Some Weeds)		
Dense Growth of Weeds	0.030	
Dense Weeds and Brush	0.040	
Swale With Grass	0.040	
	0.035	





TIME OF CONCENTRATION or TRAVEL TIME WORKSHEET

Project: Hopewell West

Designer: AJW Date: 2/22/2024

Str. No.: Pre-Development

Sh	eet	FI	low

1. Surface Description	grass	concrete	
2. Manning's Roughness Coeff., (n)	0.025	0.013	
3. Flow Length, (L) **total L<= 100 ft	70.00 ft.	30.00 ft.	ft.
4. Two-yr 24-hr Rainfall, (P2)	3.07 in.	3.07 in.	in.
5. Land Slope, (s)	0.1020 ft./ft.	0.0200 ft./ft.	ft./ft.
6. Travel Time, (Tt) (Tt = [0.007(nL)^0.8]/[P2^0.5*s^0.4])	0.016 hr	+ 0.009 hr +	0.000 hr

Shallow Concentrated Flow

7. Surface Description (paved or unpaved)	paved	paved	
8. Flow Length, (L)	460.00 ft.	310.00 ft.	ft.
9. Watercourse Slope, (s)	0.0430 ft./ft.	0.0800 ft./ft.	ft./ft.
10. Average Velocity, (V) (Vp = 20.683(s)^0.5) (Vup = 16.393(s)^0.5)	3.399 ft./s	4.637 ft./s	0.000 ft./s
11. Travel Time, (Tt) (Tt = L/3600V)	0.038 hr	+ 0.019 hr +	0.000 hr

Watershed or
Subarea Tc or Tt =

0.081 hr

or

4.84 min

Channel Flow (

12. Cross Sectional Flow Area, (a)		ft.^2			ft.^2		ft.^2
13. Wetted Perimeter, Pw		ft.			ft.		ft.
14. Hydraulic Radius, (r) (r = a/Pw)	#DIV/0!	ft.		#DIV/0!	ft.	#DIV/0!	ft.
15. Channel Slope, (s)		ft./ft.			ft./ft.		ft./ft.
16. Manning's Roughness Coeff., (n)							
17. Velocity, (V) (V = [1.486*r^(2/3)*s^(1/2)]/n)	#DIV/0!	ft./s		#DIV/0!	ft./s	#DIV/0!	ft./s
18. Flow Length, (L)		ft.			ft.		ft.
19. Travel Time, (Tt)	0.000) hr	+	0.000) hr	+ 0.00	0 hr

TIME OF CONCENTRATION or TRAVEL TIME WORKSHEET

Project: Hopewell West

Designer: AJW Date: 2/22/2024

Str. No.: Post-Development

Sheet	FI	low
-------	----	-----

1. Surface Description	grass	grass	
2. Manning's Roughness Coeff., (n)	0.025	0.025	
3. Flow Length, (L) **total L<= 100 ft	75.00 ft.	25.00 ft.	ft.
4. Two-yr 24-hr Rainfall, (P2)	3.07 in.	3.07 in.	in.
5. Land Slope, (s)	0.0050 ft./ft.	0.2500 ft./ft.	ft./ft.
6. Travel Time, (Tt) (Tt = [0.007(nL)^0.8]/[P2^0.5*s^0.4])	0.055 hr	+ 0.005 hr +	0.000 hr

Shallow Concentrated Flow

7. Surface Description (paved or unpaved)	unpaved	unpaved	
8. Flow Length, (L)	190.00 ft.	60.00 ft.	ft.
9. Watercourse Slope, (s)	0.0050 ft./ft.	0.2500 ft./ft.	ft./ft.
10. Average Velocity, (V) (Vp = 20.683(s)^0.5) (Vup = 16.393(s)^0.5)	1.463 ft./s	10.342 ft./s	0.000 ft./s
11. Travel Time, (Tt) (Tt = L/3600V)	0.036 hr	+ 0.002 hr +	0.000 hr

Watershed or
Subarea Tc or Tt =

0.118 hr
or

7.07 min

Channel Flow (

12. Cross Sectional Flow Area, (a)	12 in Pipe 0.75 ft.^2	18 in Pipe 1.68 ft.^2	24 in Pipe 2.98 ft.^2
13. Wetted Perimeter, Pw	2.50 ft.	3.75 ft.	5.00 ft.
14. Hydraulic Radius, (r) (r = a/Pw)	0.298 ft.	0.447 ft.	0.596 ft.
15. Channel Slope, (s)	0.0050 ft./ft.	0.0500 ft./ft.	0.0100 ft./ft.
16. Manning's Roughness Coeff., (n)	0.013	0.013	0.013
17. Velocity, (V) (V = [1.486*r^(2/3)*s^(1/2)]/n)	3.606 ft./s	14.941 ft./s	8.094 ft./s
18. Flow Length, (L)	145.00 ft.	300.00 ft.	105.00 ft.
19. Travel Time, (Tt)	0.011 hr	+ 0.006 hr	+ 0.004 hr

DETENTION STORAGE CALCULATIONS

	Designer: AJW	Date: 02/15/24	
Release Rate Return Period	2 yrs	Design Return Period	2 yrs
Watershed Area, (AU) (Undeveloped Watershed)	10.83 acres	Watershed Area, (AD) (Developed Watershed)	10.83 acres
Time of Concentration (Undeveloped Watershed)	5.00 min.	Developed Runoff Coefficient, (CD)	0.73
Rainfall Intensity, (iU)	5.48 in./hr		
Undeveloped Runoff Coefficient, (CU)	0.50		
Undeveloped Runoff Rate, (O) (O = CU*iU*AU)	29.67 cfs		

Storm Duration	Rainfall Intensity	Inflow Rate	Outflow Rate	Storage Rate	Required Storage
td	id	I(td) = (CD*id*AD)	O = (CU*iU*AU)	I(td)-O	[I(td)-O]*td/12
(hrs)	(in./hr)	(cfs)	(cfs)	(cfs)	(acre-ft)
0.08	5.48	43.32	29.67	13.65	0.09
0.17	4.28	33.84	29.67	4.16	0.06
0.25	3.49	27.59	29.67	-2.08	-0.04
0.50	2.34	18.50	29.67	-11.17	-0.47
1.00	1.43	11.31	29.67	-18.37	-1.53
2.00	0.84	6.64	29.67	-23.03	-3.84
3.00	0.59	4.66	29.67	-25.01	-6.25
6.00	0.36	2.85	29.67	-26.83	-13.41
12.00	0.21	1.66	29.67	-28.01	-28.01
24.00	0.13	1.03	29.67	-28.65	-57.29

DETENTION STORAGE CALCULATIONS

	Designer: AJW	Date: 06/26/23	
Release Rate Return Period	10 yrs	Design Return Period	10 yrs
Watershed Area, (AU) (Undeveloped Watershed)	10.83 acres	Watershed Area, (AD) (Developed Watershed)	10.83 acres
Time of Concentration (Undeveloped Watershed)	5.00 min.	Developed Runoff Coefficient, (CD)	0.73
Rainfall Intensity, (iU)	7.45 in./hr		
Undeveloped Runoff Coefficient, (CU)	0.50		
Undeveloped Runoff Rate, (O) (O = CU*iU*AU)	40.34 cfs		

Storm Duration	Rainfall Intensity	Inflow Rate	Outflow Rate	Storage Rate	Required Storage
td	id	I(td) = (CD*id*AD)	O = (CU*iU*AU)	I(td)-O	[I(td)-O]*td/12
(hrs)	(in./hr)	(cfs)	(cfs)	(cfs)	(acre-ft)
0.08	7.45	58.90	40.34	18.56	0.12
0.17	5.75	45.46	40.34	5.12	0.07
0.25	4.72	37.32	40.34	-3.03	-0.06
0.50	3.28	25.93	40.34	-14.41	-0.60
1.00	2.08	16.44	40.34	-23.90	-1.99
2.00	1.23	9.72	40.34	-30.62	-5.10
3.00	0.88	6.96	40.34	-33.38	-8.35
6.00	0.53	4.19	40.34	-36.15	-18.08
12.00	0.31	2.45	40.34	-37.89	-37.89
24.00	0.18	1.42	40.34	-38.92	-77.84

DETENTION STORAGE CALCULATIONS

	Designer: AJW	Date: 06/26/23	
Release Rate Return Period	100 yrs	Design Return Period	100 yrs
Watershed Area, (AU) (Undeveloped Watershed)	10.83 acres	Watershed Area, (AD) (Developed Watershed)	10.83 acres
Time of Concentration (Undeveloped Watershed)	5.00 min.	Developed Runoff Coefficient, (CD)	0.91
Rainfall Intensity, (iU)	10.42 in./hr		
Undeveloped Runoff Coefficient, (CU)	0.63		
Undeveloped Runoff Rate, (O) (O = CU*iU*AU)	70.53 cfs		

Storm Duration	Rainfall Intensity	Inflow Rate	Outflow Rate	Storage Rate	Required Storage
td	id	I(td) = (CD*id*AD)	O = (CU*iU*AU)	I(td)-O	[I(td)-O]*td/12
(hrs)	(in./hr)	(cfs)	(cfs)	(cfs)	(acre-ft)
0.08	10.42	102.69	70.53	32.16	0.21
0.17	7.84	77.27	70.53	6.74	0.10
0.25	6.49	63.96	70.53	-6.57	-0.14
0.50	4.69	46.22	70.53	-24.31	-1.01
1.00	3.13	30.85	70.53	-39.68	-3.31
2.00	1.91	18.82	70.53	-51.71	-8.62
3.00	1.39	13.70	70.53	-56.83	-14.21
6.00	0.85	8.38	70.53	-62.15	-31.08
12.00	0.47	4.63	70.53	-65.90	-65.90
24.00	0.28	2.76	70.53	-67.77	-135.54

OUTLET PRESSURE PIPE FLOW CALCULATIONS (2-Yr)

	Designer:	DJC	Date: 2/29/2024
Pipe Diameter, (D)	2.1 ft		
Entrance Loss Coef., (Ke)	0.50		50; rounded inlets = 0.10; grooved or socket-ended pipe = 0.15; grooved or socket-ends = 0.20; projecting steel or corrugated met. pipe = 0.85)
Outlet Loss Coef., (Ko)	1.0	(usually taken as 1.0)	
Manning's Rough. Coef., (n)	0.013		
Acceleration of Gravity, (g)	32.20 ft/s^2		
Length of Pipe, (L)	0.33 ft		
Top of Detention Elevation	754.04 ft		
Bottom of Detention Elevation	751.36 ft		
Height of Water, (hp) (hp = height of water above center of pipe opening)	1.63 ft^2		
Area of Pipe, (Ap) (Ap = pi(D^2)/4*2)	3.46 ft^2		
Flowrate, (Q) (see equation to right)	28.97 cfs	Q = Ap(hp/(((P	e+Ko)/2g)+(2.87n^2L/D^(4/3))))^.5

OUTLET PRESSURE PIPE FLOW CALCULATIONS (10-Yr, Top Bottom Orifice)

	Designer:	DJC	Date: 2/29/2024
Pipe Diameter, (D)	2.1 ft		
Entrance Loss Coef., (Ke)	0.50		50; rounded inlets = 0.10; grooved or socket-ended pipe = 0.15; grooved or socket-ends = 0.20; projecting steel or corrugated met. pipe = 0.85)
Outlet Loss Coef., (Ko)	1.0	(usually taken as 1.0)	
Manning's Rough. Coef., (n)	0.013		
Acceleration of Gravity, (g)	32.20 ft/s^2		
Length of Pipe, (L)	0.33 ft		
Top of Detention Elevation	754.90 ft		
Bottom of Detention Elevation	751.36 ft		
Height of Water, (hp)	2.49 ft^2		
(hp = height of water above center of pipe opening)			
Area of Pipe, (Ap) (Ap = pi(D^2)/4)	3.46 ft^2		
Flowrate, (Q) (see equation to right)	35.76 cfs	Q = Ap(hp/(((R	e+Ko)/2g)+(2.87n^2L/D^(4/3))))^.5

OUTLET PRESSURE PIPE FLOW CALCULATIONS (10-Yr, Top Orifice)

	Designer:	DJC	Date: 2/29/2024
Pipe Diameter, (D)	1.5 ft		
Entrance Loss Coef., (Ke)	0.50	` .	50; rounded inlets = 0.10; grooved or socket-ended pipe = 0.15; grooved or socket-ends = 0.20; projecting steel or corrugated met. pipe = 0.85)
Outlet Loss Coef., (Ko)	1.0	(usually taken as 1.0)	
Manning's Rough. Coef., (n)	0.013		
Acceleration of Gravity, (g)	32.20 ft/s^2		
Length of Pipe, (L)	0.33 ft		
Top of Detention Elevation	754.90 ft		
Bottom of Detention Elevation	754.04 ft		
Height of Water, (hp) (hp = height of water above center of pipe opening)	0.11 ft^2		
Area of Pipe, (Ap) (Ap = pi(D^2)/4)	1.77 ft^2		
Flowrate, (Q) (see equation to right)	3.82 cfs	Q = Ap(hp/(((K	e+Ko)/2g)+(2.87n^2L/D^(4/3))))^.5
Total Flowrate, (Qt)	39.57 cfs		

OUTLET PRESSURE PIPE FLOW CALCULATIONS (100-Yr, Top Bottom Orifice)

	Designer:	DJC	Date: 2/29/2024
Pipe Diameter, (D)	2.1 ft		
Entrance Loss Coef., (Ke)	0.50	` .	.50; rounded inlets = 0.10; grooved or socket-ended pipe = 0.15; grooved or socket-ends = 0.20; projecting steel or corrugated met. pipe = 0.85)
Outlet Loss Coef., (Ko)	1.0	(usually taken as 1.0)	
Manning's Rough. Coef., (n)	0.013		
Acceleration of Gravity, (g)	32.20 ft/s^2		
Length of Pipe, (L)	0.33 ft		
Top of Detention Elevation	757.54 ft		
Bottom of Detention Elevation	751.36 ft		
Height of Water, (hp)	5.13 ft^2		
(hp = height of water above center of pipe opening)			
Area of Pipe, (Ap) (Ap = pi(D^2)/4)	3.46 ft^2		
Flowrate, (Q) (see equation to right)	51.35 cfs	Q = Ap(hp/(((P	(e+Ko)/2g)+(2.87n^2L/D^(4/3))))^.5

OUTLET PRESSURE PIPE FLOW CALCULATIONS (100-Yr, Top Orifice)

	Designer:	DJC	Date: 2/29/2024
Pipe Diameter, (D)	1.5 ft		
Entrance Loss Coef., (Ke)	0.50	` .	50; rounded inlets = 0.10; grooved or socket-ended pipe = 0.15; grooved or socket-ends = 0.20; projecting steel or corrugated met. pipe = 0.85)
Outlet Loss Coef., (Ko)	1.0	(usually taken as 1.0)	
Manning's Rough. Coef., (n)	0.013		
Acceleration of Gravity, (g)	32.20 ft/s^2		
Length of Pipe, (L)	0.33 ft		
Top of Detention Elevation	757.54 ft		
Bottom of Detention Elevation	754.04 ft		
Height of Water, (hp) (hp = height of water above center of pipe opening)	2.75 ft^2		
Area of Pipe, (Ap) (Ap = pi(D^2)/4)	1.77 ft^2		
Flowrate, (Q) (see equation to right)	19.18 cfs	Q = Ap(hp/(((K	e+Ko)/2g)+(2.87n^2L/D^(4/3))))^.5
Total Flowrate, (Qt)	70.53 cfs		

DETENTION STORAGE CALCULATIONS (w/ OUTLET STRUCTURE)

	Designer: AJW	Date: 02/15/24	
Release Rate Return Period	2 yrs	Design Return Period	2 yrs
Watershed Area, (AU) (Undeveloped Watershed)	10.83 acres	Watershed Area, (AD) (Developed Watershed)	10.83 acres
Time of Concentration (Undeveloped Watershed)	5.00 min.	Developed Runoff Coefficient, (CD)	0.73
Rainfall Intensity, (iU)	5.48 in./hr		
Undeveloped Runoff Coefficient, (CU)	0.50		
Undeveloped Runoff Rate (= CU*iU*AU)	29.67 cfs		
Outlet Str. Outflow Rate, (O)	28.97 cfs		

Storm Duration	Rainfall Intensity	Inflow Rate	Outflow Rate	Storage Rate	Required Storage
td	id	I(td) = (CD*id*AD)	O =	I(td)-O	[I(td)-O]*td/12
(hrs)	(in./hr)	(cfs)	(cfs)	(cfs)	(acre-ft)
0.08	5.48	43.32	28.97	14.35	0.10
0.17	4.28	33.84	28.97	4.87	0.07
0.25	3.49	27.59	28.97	-1.38	-0.03
0.50	2.34	18.50	28.97	-10.47	-0.44
1.00	1.43	11.31	28.97	-17.67	-1.47
2.00	0.84	6.64	28.97	-22.33	-3.72
3.00	0.59	4.66	28.97	-24.31	-6.08
6.00	0.36	2.85	28.97	-26.12	-13.06
12.00	0.21	1.66	28.97	-27.31	-27.31
24.00	0.13	1.03	28.97	-27.94	-55.89

DETENTION STORAGE CALCULATIONS (w/ OUTLET STRUCTURE)

	Designer: AJW	Date: 06/26/23	
Release Rate Return Period	10 yrs	Design Return Period	10 yrs
Watershed Area, (AU) (Undeveloped Watershed)	10.83 acres	Watershed Area, (AD) (Developed Watershed)	10.83 acres
Time of Concentration (Undeveloped Watershed)	5.00 min.	Developed Runoff Coefficient, (CD)	0.73
Rainfall Intensity, (iU)	7.45 in./hr		
Undeveloped Runoff Coefficient, (CU)	0.50		
Undeveloped Runoff Rate (= CU*iU*AU)	40.34 cfs		
Outlet Str. Outflow Rate, (O)	39.57 cfs		

Storm Duration	Rainfall Intensity	Inflow Rate	Outflow Rate	Storage Rate	Required Storage
td	id	I(td) = (CD*id*AD)	O =	I(td)-O	[I(td)-O]*td/12
(hrs)	(in./hr)	(cfs)	(cfs)	(cfs)	(acre-ft)
0.08	7.45	58.90	39.57	19.33	0.13
0.17	5.75	45.46	39.57	5.89	0.08
0.25	4.72	37.32	39.57	-2.25	-0.05
0.50	3.28	25.93	39.57	-13.64	-0.57
1.00	2.08	16.44	39.57	-23.13	-1.93
2.00	1.23	9.72	39.57	-29.85	-4.97
3.00	0.88	6.96	39.57	-32.61	-8.15
6.00	0.53	4.19	39.57	-35.38	-17.69
12.00	0.31	2.45	39.57	-37.12	-37.12
24.00	0.18	1.42	39.57	-38.15	-76.29

DETENTION STORAGE CALCULATIONS (w/ OUTLET STRUCTURE)

	Designer: AJW	Date: 06/26/23	
Release Rate Return Period	100 yrs	Design Return Period	100 yrs
Watershed Area, (AU) (Undeveloped Watershed)	10.83 acres	Watershed Area, (AD) (Developed Watershed)	10.83 acres
Time of Concentration (Undeveloped Watershed)	5.00 min.	Developed Runoff Coefficient, (CD)	0.91
Rainfall Intensity, (iU)	10.42 in./hr		
Undeveloped Runoff Coefficient, (CU)	0.63		
Undeveloped Runoff Rate, (O) (O = CU*iU*AU)	70.53 cfs		

Storm Duration	Rainfall Intensity	Inflow Rate	Outflow Rate	Storage Rate	Required Storage
td	id	I(td) = (CD*id*AD)	O = (CU*iU*AU)	I(td)-O	[I(td)-O]*td/12
(hrs)	(in./hr)	(cfs)	(cfs)	(cfs)	(acre-ft)
0.08	10.42	102.69	70.53	32.16	0.21
0.17	7.84	77.27	70.53	6.74	0.10
0.25	6.49	63.96	70.53	-6.57	-0.14
0.50	4.69	46.22	70.53	-24.31	-1.01
1.00	3.13	30.85	70.53	-39.68	-3.31
2.00	1.91	18.82	70.53	-51.71	-8.62
3.00	1.39	13.70	70.53	-56.83	-14.21
6.00	0.85	8.38	70.53	-62.15	-31.08
12.00	0.47	4.63	70.53	-65.90	-65.90
24.00	0.28	2.76	70.53	-67.77	-135.54

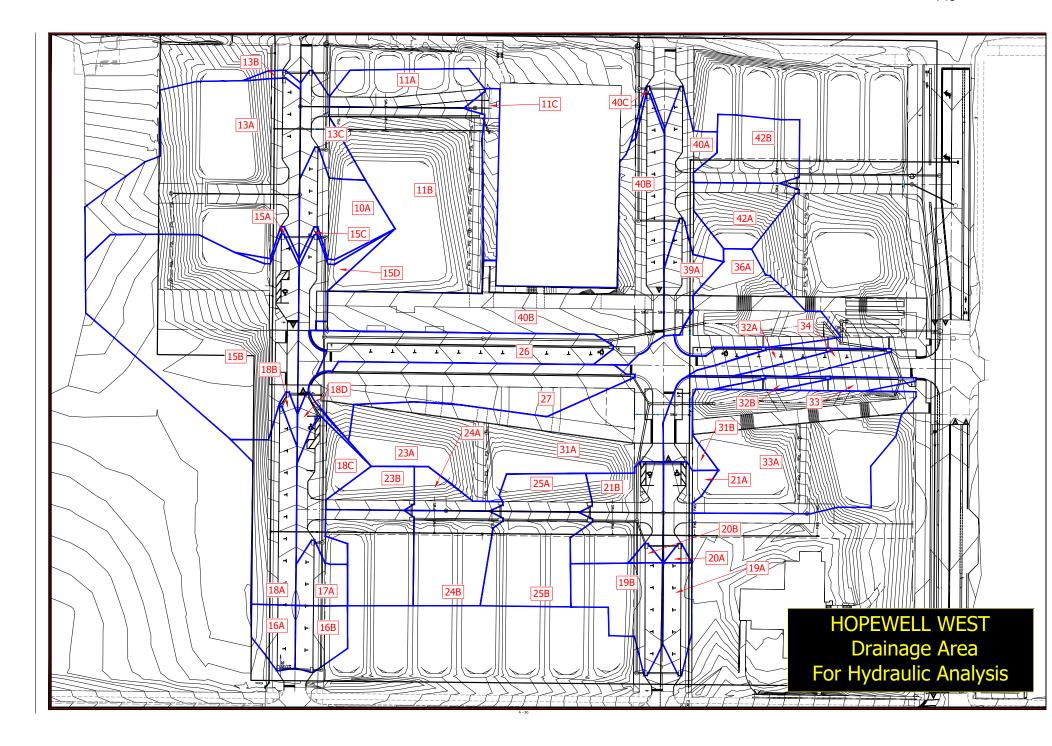


Aqua-Swirl™ Model	Swirl Chamber Diameter	Maximum Stub-Out Pipe Outer Diameter		Water Quality Treatment Flow ²	Oil/Debris Storage Capacity	Sediment Storage Capacity
	(ft.)		1.)	(cfs)	(gal)	(ft³)
AS-2	2.50	On/Offline 8	BYP ¹ 15	1.1	37	10
AS-3	3.25	10	21	1.8	110	20
AS-4	4.25	12	27	3.2	190	32
AS-5	5.00	12	30	4.4	270	45
AS-6	6.00	14	36	6.3	390	65
AS-7	7.00	16	42	8.6	540	90
AS-8	8.00	18	48	11.2	710	115
AS-9	9.00	20	>48 *	14.2	910	145
AS-10	10.0	22	>48 *	17.5	1130	180
AS-11	11.0	24	>48 *	21.2	1422	222
AS-12	12.0	26	>48 *	25.2	1698	270
AS-13	13.0	28	>48 *	29.6	1986	310
AS-XX	Custom			>26 **		

^{*} See Representative for larger pipe diameters available **Higher water quality treatment flow rates can be designed with multiple swirls.

- The Aqua-SwirI™ Internal Bypass (BYP) provides full treatment of the "first flush," while the peak design storm is diverted and channeled through the main conveyance pipe. Please refer to your local representative for more information.
- 2) Many regulatory agencies are establishing "water quality treatment flow rates" for their areas based on the initial movement of pollutants into the storm drainage system. The treatment flow rate of the Aqua-Swirl™ system is engineered to meet or exceed the local water quality treatment criteria. This "water quality treatment flow rate" typically represents approximately 90% to 95% of the total annual runoff volume.

The design and orientation of the Aqua-Filter™ generally entails some degree of customization. For assistance in design and specific sizing using historical rainfall data, please refer to an AquaShield™ representative or visit our website at www.AquaShieldInc.com. CAD details and specifications are available upon request.



			Hopewell Wo	est - Proposed	Drainage Areas	for Q10			
In	npervious Runof	f Coefficient, "(C" =	0.82					
I	Pervious Runoff	Coefficient, "C"	'=	0.21					
	Proposed	Proposed	Undeveloped	Undeveloped	i				
Structure	Impervious Area	Pervious Area	Impervious Area	Pervious Area	Impervious Area	Pervious Area	Total Area	Total Area	Composite "C
Number	(SFT)	(SFT)	(SFT)	(SFT)	(SFT)	(SFT)	(SFT)	(Acres)	1 '
10A	2015	397	3294	581	5309	978	6287	0.14	0.73
11A	1906	0	3105	548	5011	548	5559	0.13	0.76
11B	2707	725	17258	3045	19965	3770	23735	0.54	0.72
11C	726	2229	0	0	726	2229	2955	0.07	0.36
13A	5163	5492	14958	2640	20121	8132	28253	0.65	0.64
13B	189	35	122	21	311	56	367	0.01	0.73
13C	1881	549	1018	180	2899	729	3628	0.08	0.70
15A	180	80	70	12	250	92	342	0.01	0.66
15B	3916	14975	12081	2132	15997	17107	33104	0.76	0.50
15C	178	58	141	25	319	83	402	0.01	0.69
15D	3204	606	674	119	3878	725	4603	0.11	0.72
16A	1603	965	0	0	1603	965	2568	0.06	0.59
16B	1617	315	1056	186	2673	501	3174	0.07	0.72
17A	1398	247	855	151	2253	398	2651	0.06	0.73
18A	4540	3891	0	0	4540	3891	8431	0.19	0.54
18B	421	46	0	0	421	46	467	0.01	0.76
18C	2619	1544	1567	277	4186	1821	6007	0.14	0.64
18D	788	79	122	22	910	101	1011	0.02	0.76
19A	2376	533	0	0	2376	533	2909	0.07	0.71
19B	2440	514	3108	548	5548	1062	6610	0.15	0.72
20A	250	71	0	0	250	71	321	0.01	0.69
20B	282	87	21	4	303	91	394	0.01	0.68
21A	2549	260	428	76	2977	336	3313	0.08	0.76
21B	2137	1632	3359	593	5496	2225	7721	0.18	0.64
23A	893	0	1963	346	2856	346	3202	0.07	0.75
23B	893	0	5424	957	6317	957	7274	0.17	0.74
24A	910	0	1100	194	2010	194	2204	0.05	0.77
24B	910	0	5594	987	6504	987	7491	0.17	0.74
25A	926	0	2163	382	3089	382	3471	0.08	0.75
25B	926	0	6406	1131	7332	1131	8463	0.19	0.74
26	8991	322	0	0	8991	322	9313	0.21	0.80
27	8184	5780	0	0	8184	5780	13964	0.32	0.57
31A	9744	2249	12742	2248	22486	4497	26983	0.62	0.72
31B	2584	163	403	71	2987	234	3221	0.07	0.78
32A	1810	0	0	0	1810	0	1810	0.04	0.82
32B	914	0	0	0	914	0	914	0.02	0.82
33A	5094	4266	12184	2150	17278	6416	23694	0.54	0.65
33	1319	0	0	0	1319	0	1319	0.03	0.82
34	1833	0	0	0	1833	0	1833	0.04	0.82
36A	3737	3457	2416	426	6153	3883	10036	0.23	0.58
39A	2406	341	571	101	2977	442	3419	0.08	0.74
40A	3196	1707	655	116	3851	1823	5674	0.13	0.62
40B	12775	12000	0	0	12775	12000	24775	0.57	0.52
40C	204	93	0	0	204	93	297	0.01	0.63
42A	1053	0	3477	613	4530	613	5143	0.12	0.75
42B	1076	0	4270	753	5346	753	6099	0.14	0.74
420	10/0	ı U	42/0	/ / / / /	3340	/33	בבטט	0.14	0.74

			Hopewell We	st - Proposed I	Orainage Areas	for Q100				
Ir	npervious Runof	f Coefficient, "C	<u>;"</u> =	1.03						
	Pervious Runoff	Coefficient. "C"	=	0.26						
	Proposed	Proposed	Undeveloped	Undeveloped						
Structure	Impervious Area	Pervious Area	Impervious Area	Pervious Area	Impervious Area	Pervious Area	Total Area	Total Area	Composite "C	
Number	(SFT)	(SFT)	(SFT)	(SFT)	(SFT)	(SFT)	(SFT)	(Acres)		
10A	2015	397	3294	581	5309	978	6287	0.14	0.91	
11A	1906	0	3105	548	5011	548	5559	0.13	0.95	
11B	2707	725	17258	3045	19965	3770	23735	0.54	0.91	
11C	726	2229	0	0	726	2229	2955	0.07	0.45	
13A	5163	5492	14958	2640	20121	8132	28253	0.65	0.81	
13B	189	35	122	21	311	56	367	0.01	0.91	
13C	1881	549	1018	180	2899	729	3628	0.08	0.88	
15A	180	80	70	12	250	92	342	0.01	0.82	
15B	3916	14975	12081	2132	15997	17107	33104	0.76	0.63	
15C	178	58	141	25	319	83	402	0.01	0.87	
15D	3204	606	674	119	3878	725	4603	0.11	0.91	
16A	1603	965	0	0	1603	965	2568	0.06	0.74	
16B	1617	315	1056	186	2673	501	3174	0.07	0.91	
17A	1398	247	855	151	2253	398	2651	0.06	0.91	
18A	4540	3891	0	0	4540	3891	8431	0.19	0.67	
18B	421	46	0	0	421	46	467	0.01	0.95	
18C	2619	1544	1567	277	4186	1821	6007	0.14	0.80	
18D	788	79	122	22	910	101	1011	0.02	0.95	
19A	2376	533	0	0	2376	533	2909	0.07	0.89	
19B	2440	514	3108	548	5548	1062	6610	0.15	0.91	
20A	250	71	0	0	250	71	321	0.01	0.86	
20B	282	87	21	4	303	91	394	0.01	0.85	
21A	2549	260	428	76	2977	336	3313	0.08	0.95	
21B	2137	1632	3359	593	5496	2225	7721	0.18	0.81	
23A	893	0	1963	346	2856	346	3202	0.07	0.95	
23B	893	0	5424	957	6317	957	7274	0.17	0.93	
24A	910	0	1100	194	2010	194	2204	0.05	0.96	
24B	910	0	5594	987	6504	987	7491	0.17	0.93	
25A	926	0	2163	382	3089	382	3471	0.08	0.95	
25B	926	0	6406	1131	7332	1131	8463	0.19	0.93	
26	8991	322	0	0	8991	322	9313	0.21	1.00	
27	8184	5780	0	0	8184	5780	13964	0.32	0.71	
31A	9744	2249	12742	2248	22486	4497	26983	0.62	0.90	
31B	2584	163	403	71	2987	234	3221	0.07	0.97	
32A	1810	0	0	0	1810	0	1810	0.04	1.03	
32B	914	0	0	0	914	0	914	0.02	1.03	
33A	5094	4266	12184	2150	17278	6416	23694	0.54	0.82	
33	1319	0	0	0	1319	0	1319	0.03	1.03	
34	1833	0	0	0	1833	0	1833	0.04	1.03	
36A	3737	3457	2416	426	6153	3883	10036	0.23	0.73	
39A 40A	2406 3196	341 1707	571 655	101 116	2977 3851	442 1823	3419 5674	0.08 0.13	0.93 0.78	
40B	12775 204	12000	0	0	12775 204	12000	24775	0.57 0.01	0.66	
40C	1053	93 0	3477	613	4530	93 613	297 5143	0.01	0.79 0.94	
42A 42B		_		753						
42B	2B 1076 0 4270				5346	753	6099	0.14	0.93	

TIME OF CONCENTRATION or TRAVEL TIME WORKSHEET

Project: Hopewell West

Designer: DJC Date: 2/26/2024

Str 22 to Str 33A

Sheet Flow						
1. Surface Description						
2. Manning's Roughness Coeff., (n)						
3. Flow Length, (L) **total L<= 100 ft	ft.			ft.		ft.
4. Two-yr 24-hr Rainfall, (P2)	in.			in.		in.
5. Land Slope, (s)	ft./ft.			ft./ft.		ft./ft.
6. Travel Time, (Tt) (Tt = [0.007(nL)^0.8]/[P2^0.5*s^0.4])	0.000 hr	+	0.000	hr +	0.000	hr
Shallow Concentrated Flow						

7. Surface Description (paved or unpaved)

8. Flow Length, (L)

(paved or unpaved)

ft.

0.000 hr

ft.

ft.

9. Watercourse Slope, (s)

ft./ft.

ft./ft.

ft./ft.

10. Average Velocity, (V) (Vp = 20.683(s)^0.5)

 $(Vup = 16.393(s)^0.5)$

0.000 ft./s

0.000 ft./s

0.000 hr

0.000 ft./s

0.000 hr

Subarea Tc or Tt =

Watershed or

0.016 hr or 0.95 min

Channel Flow

11. Travel Time, (Tt)

(Tt = L/3600V)

12. Cross Sectional Flow Area, (a)	Existing E	Earth ft.^2	Pipe	ft.^2	Rock Channel 78.00 ft.^2
13. Wetted Perimeter, Pw		ft.		ft.	31.80 ft.
14. Hydraulic Radius, (r) (r = a/Pw)	#DIV/0!	ft.	#DIV/0!	ft.	2.453 ft.
15. Channel Slope, (s)		ft./ft.		ft./ft.	0.0020 ft./ft.
16. Manning's Roughness Coeff., (n)					0.035
17. Velocity, (V) (V = [1.486*r^(2/3)*s^(1/2)]/n)	#DIV/0!	ft./s	#DIV/0!	ft./s	3.453 ft./s
18. Flow Length, (L)		ft.		ft.	196.80 ft.
19. Travel Time, (Tt) (Tt = L/3600V)	0.000	hr +	0.000	hr +	0.016 hr

P2 is from NOAA Precipitation Depths Table in Appendices Manning's n from IDM Figures

TIME OF CONCENTRATION or TRAVEL TIME WORKSHEET

Project: Hopewell West

Designer: DJC Date: 2/26/2024

Str 30 to Str 36A

Sheet Flow

1.	Surface	Description

- 2. Manning's Roughness Coeff., (n)
- 3. Flow Length, (L) **total L<= 100 ft
- 4. Two-yr 24-hr Rainfall, (P2)
- 5. Land Slope, (s)
- 6. Travel Time, (Tt) $(Tt = [0.007(nL)^0.8]/[P2^0.5*s^0.4])$
- ft./ft.
 - 0.000 hr

ft.

in.

- 0.000 hr
- ft./ft. 0.000 hr
- ft./ft.

ft.

in.

Shallow Concentrated Flow

- 7. Surface Description (paved or unpaved)
- 8. Flow Length, (L)

- ft.
- ft.

0.000 ft./s

ft.

in.

ft.

9. Watercourse Slope, (s)

ft./ft.

0.000 ft./s

- ft./ft.
- ft./ft.

0.000 ft./s

- 10. Average Velocity, (V) $(Vp = 20.683(s)^0.5)$

 - $(Vup = 16.393(s)^0.5)$
- 11. Travel Time, (Tt) (Tt = L/3600V)

0.000 hr

0.000 hr

0.010 hr or 0.60 min

Watershed or

Subarea Tc or Tt =

Channel Flow

12. Cross Sectional Flow Area, (a	ı)
-----------------------------------	----

Existing Earth ft.^2 Pipe

Rock Channel 78.00 ft.^2

13. Wetted Perimeter, Pw

- ft.
- ft.

ft.^2

31.80 ft.

- 14. Hydraulic Radius, (r) (r = a/Pw)
- #DIV/0! ft.
- #DIV/0! ft.
- 2.453 ft.

15. Channel Slope, (s)

17. Velocity, (V)

- ft./ft.
- ft./ft.
 - 0.035

16. Manning's Roughness Coeff., (n)

 $(V = [1.486*r^{2/3})*s^{1/2}]/n)$

- #DIV/0! ft./s
- #DIV/0! ft./s
- 3.453 ft./s

0.0020 ft./ft.

18. Flow Length, (L)

- ft.
- 123.90 ft. ft.

19. Travel Time, (Tt) (Tt = L/3600V)

0.010 hr 0.000 hr 0.000 hr

P2 is from NOAA Precipitation Depths Table in Appendices Manning's n from IDM Figures

Computed By: DJC Inlet Comp Worksheet Project Hopewell West Date 2/21/2023

Pavement Roughness Coefficient - n = 0.013 Adjacent Lane Width = Varies Gutter width = 0 ft d-d = depth at curb face d-ce = depth at casting edge Inlet Intake Formulas: for slopes equal or greater than 1% the Neenah method is used; for slopes less than 1% the Weir equation method is used FA = Area of Flow over Casting's width

Depressed Curb Inlet Weir Coefficient (Cw) = 2.3

												3.2 (Curb In	the Weir ed lets)	quation met	ilou is useu		I A - Alea	of Flow over C	asung s wi	iuui		15		3.48									_		
Location			Gutter Disc	charge F	Decian Ere	auenau = 1	In ve					Gutter F	ischarge A	Uloumble Si	read = Var	ine (enn pla	ine)			Inlet Informa	lion	_		te/Combinatio lition, SL <1%	n Inlets O	nly SL>1%	1 0	n Grade	1 0		Curb Tur	rnouts Only Runby	i		
Inlet no Station	DA - Impi	er C	DA - grass			Comp		Intensity	Q=CiA	SL	ST		Tot. Gutter					V			Length of	FA	Qi (cfs)	Qi (cfs)	Qwi (cfs)	Qni			P	Qi	Qi	Qr	Inlet	Allowable	Notes
	(sqft)	pave	(sqft)	grass	-	Coeff (C)				(%)	(S) (%)		Flow (cfs)		d-cf (ft)			(ft/s)			Opening (ft)			Cast Length		To	(=:=/		(ft)	(cfs)	(cfs)	(cfs)		Spread (ft)	
1 2 Fairview W Curb	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28 2	30	31	32	33	34	35	36	37	38
16A	1603	0.82	965	0.21	0.06	0.59	5.00	7.45	0.26	1.00%	2.00%	0.00	0.26	4.0	0.08	0.04	0.16	1.60	15	1.83	3.48					0.2	3 0.23	0.03					16A	16.5	
18A 18B	4540	0.82			0.19	0.54						0.00	0.78	4.5	0.09	0.05	0.20	3.85	15	1.83	3.48						1 0.61						18A	16.5	
18B 15B	421 15997	0.82	46 17107	0.21	0.01	0.76	5.00	7.45	0.06 2.86	5.00%	2.00%	0.16 0.02	0.22 2.87	2.8 7.5	0.06	0.02	0.08	2.82 5.05	15 15	1.83	3.48					1.6	1 0.21 5 1.65	1 23		-			18B 15B	7 16.5 16.5	
15A 13A	250	0.82	92 8132	0.21	0.01	0.66	5.00	7.45	0.04	4.31%	2.00%	1.23	1.26	5.5 8.9	0.11	0.07	0.31	4.12 4.37	15 15		3.48 3.48					1 0.8	9 I 0.89	0.38		=			15A 13A	16.5 10.5	=
13B	311	0.82	56	0.21	0.01	0.73	5.00	7.45	0.05	2.57%	2.00%	1.63	1.68	6.8	0.14	0.10	0.46	3.64	15	1.83	3.48					1.3	7 1.17	0.51					13B	10.5	=
Fairview E Curb																																			
16B	2673	0.82	501			0.72						0.00	0.39	4.7	0.09	0.06	0.22	1.78	15	1.83	3.48	\vdash			-			0.06	-	\vdash			16B	16.5	-
17A 18C	2253 4186	0.82 0.82	398 1821	0.21	0.06 0.14	0.73 0.64	5.00	7.45	0.33	5.00%	2.00%	0.00	0.33 0.68	3.3 4.3	0.07	0.03	0.11	3.11 3.73	15 15	1.83	3.48	-			=	0.3	0 0.30 5 0.55	0.03 0.14	-	=			17A 18C	16.5 16.5	=
18C 18D 15D	910	0.82	101	0.21	0.02	0.76	5.00	1 7.45	I 0.13	5.00%	1 2.00%	0.14	0.27	3.0	0.06	0.02	0.09	2.95 3.40	15	1.83	3.48					0.3	5 0.25	0.02					18D 15D	7	=
15C	319	0.82	83	0.21	0.01	0.69	5.00	7.45	0.05	4.31%	2.00%	0.00	0.05	1.6	0.03	0.00	0.03	1.81	15	1.83	3.48					0.3	2 0.32	0.00					15C 10A	16.5	
10A 13C	5309 2899	0.82	978 729	0.21	0.14	0.73	5.00	7.45	0.78	2.57%	2.00%	0.00	0.78 0.59	5.1 4.6	0.10	0.07	0.26 0.21	3.01 2.81	15 15	1.83	3.48 3.48	\vdash			-	0.6	2 0.62 1 0.51	0.16	+	\vdash			10A 13C	16.5 10.5	
S Jackson E Curb																																			
19A	2376	0.82	533	0.21	0.07	0.71	5.00	7.45	0.35	4.39%	2.00%	0.00	0.35	3.4	0.07	0.03	0.12	3.01	15	1.83	3.48				=	0.3	1 0.31	0.04					19A	16.5	=
20A	250	0.82	71	0.21	0.01	0.69	5.00	7.45	0.04	1.08%	2.00%	0.00	0.04 0.43	1.9 4.5	0.04	0.00	0.04	1.02 2.13	15 15		3.48 3.48					0.0	4 0.04	0.00					20A 21A	16.5 16.5	=
21A		0.82	336	0.21	0.08	0.76	5.00	7.45	0.43	1.53%	2.00%	0.00	0.43	4.5	0.09	0.05	0.20	2.13	15	1.83	3.48					0.0	0.37	0.06					ZIA	10.5	
S Jackson W Curl	5548	0.82	1062	0.21	0.15	0.72	5.00	7.45	0.82	4.39%	2.00%	0.00	0.82	4.7	0.09	0.06	0.22	3.72	15	1.83	3.48					0.6	3 0.63	0.18					19B	16.5	
20B	303	0.82	91	0.21	0.01	0.68	5.00	7 45	0.05	1.08%	2 00%	0.00	0.05	2.1	0.04	0.00	0.04	1.07	15	1.83	3.48	 			-	0.0	5 0.05	0.00		\vdash			20B	16.5	
21B	5496				0.18					1.53%			0.85	5.8	0.12	0.08	0.34	2.53	15	1.83	3.48						3 0.63						21B	16.5	=
N Jackson W Curl	b				0.53		5.00	7.45	0.00	0.440/	0.000/	0.00	0.00	6.4	0.40	0.00	0.40	5.51	1.	4.00	0.40							0.00					40B	16.5	
40B 40A	204		93	0.21	0.57	0.52		7.45	0.03	6.44%	2.00%	0.00 0.82	2.22 0.85	4.4	0.13	0.09 0.05	0.40	4.34	15 15	1.83	3.48 3.48					0.6	7 0.67	0.82 0.18					40B 40A	16.5	
N Jackson E Curl	b																																		
39A 40C	2977 3851	0.82	442 1823	0.21	0.08	0.74	5.00	7.45 7.45	0.43	6.44%	2.00%	0.00 0.05	0.43 0.66				0.12 0.16			1.83						0.3	8 0.38 4 0.54	0.05 0.12		=					=
Univerity N Curb		0.02	1020	0.21	-	0.02	-				-						-											-							_
26		0.82	322	0.21	0.21	0.80	5.00	7.45	1.27	2.53% 8.50%	1.33%		1.27 4.07	7.9	0.11	0.08	0.42	3.05	15	1.83	3.48 3.48					0.5	2 0.52	0.75 2.10					26 31A	9.5	
26 31A 32A		0.82 0.82	0	0.21	0.62	0.82	5.00	7 45	0.25	8 50%	2 00%	2 10	2.36		0.15 0.12	0.11	0.57 0.38	7.11 6.21	15		3.48					1.3	3 1.33	1.03					32A	9.5 9.5	
34	1833	0.82	0	0.21	0.04	0.82	5.00	7.45	0.26	8.50%	2.00%	1.03	1.28	4.9	0.10	0.06	0.24	5.33	15	1.83	3.48	\vdash				0.7	6 0.76	0.52	+	-	_		34	9.5	
Univerity S Curb	0104	0.92	5790	0.21	0.32	0.57	5.00	7.45	1 36	2 53%	1 33%		1.36	8.1	0.11	0.08	0.44	3.10	15	1.83	3.48					0.6	3 0.63	0.73					27	9.5	
27 31B 32B	2987	0.82	234	0.21	0.07	0.78	5.00	7.45	0.43	8.50%	2.00%	0.73	1.15	4.7	0.09	0.06	0.22	5.19 4.89		1.83	3.48					0.3	8 0.38	0.78 0.84					31B	9.5 9.5	
32B 33	914 1319	0.82 0.82	0	0.21	0.02	0.82	5.00	7.45	0.13	8.50%	2.00%	0.78 0.84	1.02		0.09			5.04		1.83						0.0	3 0.03	0.84						9.5	
NE Alley																																			
42B	5346	0.82			1		1	I	1	I .		0.00		4.3	0.09	0.05	0.18	4.30	15	1.83	3.48	-						0.15	+ -	\vdash			42B	6	-
42A	4530	0.82	613	0.21	0.12	0.75	5.00	7.45	0.66	6.68%	2.00%	0.00	0.66	4.0	0.08	0.04	0.16	4.12	15	1.83	3.48					0.5	5 0.55	0.11					42A	6	
SW Alley	2056	0.02	246	0.21	0.07	0.75	5.00	7.45	0.41	2 25%	2.00%	0.00	0.41	2.0	0.08	0.04	0.15	2 90	15	1.02	2.49					0.2	5 0.35	0.06					224	6	
23A 24A 25A	2010	0.82	194	0.21	0.05	0.77	5.00	7.45	0.29	3.25%	2.00%	0.06	0.35	3.6	0.07	0.04	0.13	2.80 2.68 2.92	15	1.83 1.83	3.48					0.3	1 0.31	0.06 0.04 0.07					23A 24A	6	
∠5A	l l				1		1	I	1	I .					l	l .		2.92		1.83	3.48								+				25A	б	$\overline{}$
23B 24B	6317 6504	0.82	957 987	0.21	0.17	0.74	5.00	7.45	0.92	3.25%	2.00%	0.00 0.24 0.35	0.92 1.18	5.2 5.7	0.10 0.11	0.07 0.08	0.27	3.42 3.64	15 15	1.83	3.48 3.48					0.6	9 0.69 3 0.83	0.24		=			23B 24B	6	==
25B	7332	0.82	1131	0.21	0.19	0.74	5.00	7.45	1.07	3.25%	2.00%	0.35	1.42	6.1	0.12	0.09	0.32 0.37	3.81	15	1.83 1.83	3.48					0.9	6 0.96	0.35 0.46			_		25B	6	2
NW Alley	10005	0.00	2770	0.24	0.54	0.72	5.00	7.4F	2.04	4 0007	2.00%	0.00	2.94	77	0.15	0.12	0.59	4.09	15	1.83	3.48					1.	2 1.42	1.54					110	10.5	
11B												0.00		7.7	0.15	0.12		4.98	15														11B	10.5	
11A	2899	0.82	729	0.21	0.08	0.70	5.00	7.45	0.43	4.08%	2.00%	0.00	0.43	3.7	0.07	0.04	0.14	3.08	15	1.83	3.48	\vdash			\dashv	0.3	7 0.37	0.06	+	\vdash	$-\mp$		11A	10.5	-
	•									-									•			•							•						

NOTES:

Local street spread limit requires 8.0' travel lane (width/2-4) Structure X = 41/2 - 4 = 16flt.
Structure X = 29/2 - 4 = 10.5ft.
Structure X = 27/2 - 4 = 9.5ft.
Structure X = 27/2 - 4 = 7ft.
Structure X = 20/2 - 4 = 5ft.

Spread exeeds calculated limit for Structure 25B. However, Spread from 25A is less than the limit allowing a travel lane of 8ft between the inlets.

Route:		Hopewel	Hopewell West Conc. Pipe Manning's n = 0.013 Project: Hopewell West						Computed By: DJC Date: 4/19/2024														
Lo	ocation										Proposed	Pipe Hydraulic	s										
Str. No.	Station	Structure Type	Pipe Diameter (in.)	Pipe Length (ft)	Weighted C	A (ac.)	CA	CA Previous	CA Total	Time in Pipe (min.)	Time of Concentration, Tc (min.)	Rain Intensity, I10 (in./h)	Q10 = CIA (cfs)	Rain Intensity, I100 (in./h)	Q100 = CIA (cfs)	Pipe Slope (%) Vmax (fps)	Qmax (cfs)	Q10 Eff. (%)	Q100 Eff. (%)	Upstream Elev.	Elev.	Rim Elev.	Contributes to Structure
1 11A	2	3 Inlet	12	5.0	6a	6b	6 0.097	7	0.097	9a	9b	10	11	12	13 1.26	14 15	16	17 29%	17a 50%	18	19	20	21 11B
11A 11C		Inlet	12	25.6 19.4	0.7599	0.1276 0.0678	0.097		0.097	0.13	5.00	7.45 7.45	0.72 0.18	10.42 10.42	0.32	0.50% 3.22 4.50% 9.65	2.53 7.58	29%	4%	767.12 769.30	767.00 768.43	770.12 771.95	11B 11B
11B		Inlet	15	159.3	0.7231	0.5449	0.394	0.121	0.515	0.71	5.13	7.40	3.82	10.35	6.67	0.50% 3.73	4.58	83%	146%	767.00	766.20	770.31	12
10A		Inlet	12	9.4	0.7251	0.1443	0.105		0.105	0.05	5.00	7.45	0.78	10.42	1.36	0.50% 3.22	2.53	31%	54%	774.61	774.57	777.61	10
10 12		Manhole Manhole	12 15	35.0 43.3			0.000	0.105 0.620	0.105 0.620	0.09	5.05 5.84	7.43 7.16	0.78 4.44	10.39 9.98	1.36 7.74	2.00% 6.43 0.50% 3.73	5.05 4.58	15% 97%	27% 169%	774.57 765.20	773.87 764.33	778.21 777.09	12
13A		Inlet	15	8.0	0.6444	0.6486	0.418	0.020	0.418	0.04	5.00	7.45	3.11	10.42	5.44	0.50% 3.73	4.58	68%	119%	772.04	772.00	775.19	13B
13B		Inlet	15	27.7	0.7269	0.0084	0.006	0.418	0.424	0.09	5.04	7.44	3.15	10.40	5.51	1.00% 5.28	6.48	49%	85%	771.90	771.62	775.00	13C
13C		Inlet Manhole	15 15	16.1 29.6	0.6974	0.0833	0.058	0.424 1.102	0.482 1.102	0.05	5.12 6.04	7.41 7.10	3.57 7.82	10.36 9.88	6.24	1.00% 5.28 2.00% 7.46	6.48 9.16	55% 85%	96% 149%	771.52 764.33	771.36 763.74	775.14 775.85	13 WQU1
WQU1		Manhole	18	26.4			0.000	1.102	1.102	0.07	6.10	7.07	7.80	9.85	13.57	1.00% 5.96	10.53	74%	129%	763.74	763.48	772.87	14
15A		Inlet	12	8.0	0.6559	0.0079	0.005		0.005	0.04	5.00	7.45	0.04	10.42	0.07	0.50% 3.22	2.53	2%	3% 110%	778.17	778.13	781.07	15B
15B 15C		Inlet	15 12	34.2 8.0	0.5048	0.7600	0.384	0.005	0.389	0.15	5.04	7.44 7.45	2.89 0.05	10.40	5.05 0.08	0.50% 3.73 1.00% 4.55	4.58 3.57	63% 1%	2%	778.13 778.17	777.96 778.09	781.41 781.07	15D 15D
15D		Inlet	15	9.4	0.7239	0.1057	0.076	0.395	0.472	0.04	5.19	7.38	3.48	10.32	6.08	0.50% 3.73	4.58	76%	133%	777.96	777.91	781.41	15
15		Manhole	15	101.9	0.5000	0.0500	0.000	0.472	0.472	0.46	5.24	7.37	3.48	10.30	6.07	0.50% 3.73	4.58	76%	133% 18%	777.91	777.40	782.01	28
16A 16B		Inlet	12	34.6 9.3	0.5908	0.0590 0.0729	0.035	0.035	0.035	0.18	5.00 5.18	7.45 7.39	0.26	10.42	0.45 1.13	0.50% 3.22 0.50% 3.22	2.53	10% 26%	18% 45%	791.15 790.97	790.97 790.93	794.15 794.13	16B 16
16		Manhole	12	130.0			0.000	0.088	0.088	0.67	5.23	7.37	0.65	10.30	1.13	0.50% 3.22	2.53	26%	45%	790.93	790.28	794.74	17
17A		Inlet	12	9.3	0.7284	0.0609	0.044		0.044	0.05	5.00	7.45	0.33	10.42	0.58	0.50% 3.22	2.53	13%	23%	790.48	790.44	793.48	17
17 18A		Manhole Inlet	12	151.2 16.3	0.5385	0.1935	0.000	0.132	0.132	0.26	5.90 5.00	7.14 7.45	0.94	9.96 10.42	1.64	4.50% 9.65 2.00% 6.43	7.58 5.05	12% 15%	22%	789.98 783.68	783.17 783.36	794.09 786.76	18 18B
18B		Inlet	12	21.0	0.7599	0.0107	0.008	0.104	0.112	0.11	5.04	7.44	0.84	10.40	1.46	0.50% 3.22	2.53	33%	58%	783.36	783.25	786.41	18D
18C		Inlet	12	27.0	0.6351	0.1379	0.088		0.088	0.06	5.00	7.45	0.65	10.42	1.14	3.00% 7.88	6.19	11%	18%	784.06	783.25	787.14	18D
18D 18		Inlet Manhole	12	16.0 59.3	0.7591	0.0232	0.018	0.200	0.218	0.08	5.15 6.16	7.40 7.05	1.61 2.47	10.34 9.82	2.81 4.29	0.50% 3.22 0.60% 3.52	2.53	64% 89%	111% 155%	783.25 782.17	783.17 781.82	786.31 786.19	18 28
28		Manhole	18	162.7			0.000	0.821	0.821	0.64	6.44	6.96	5.71	9.68	9.93	0.50% 4.21	7.45	77%	133%	777.40	776.59	785.89	29
29		Manhole	18	130.5			0.000	0.821	0.821	0.36	7.09	6.74	5.53	9.34	9.59	1.00% 5.96	10.53	53%	91%	776.59	775.28	782.12	30
26 30		Inlet Manhole	12 18	7.1 96.7	0.7989	0.2138	0.171	0.992	0.171 0.992	0.03	5.00 7.45	7.45 6.62	1.27 6.56	10.42 9.16	2.22 11.35	1.00% 4.55 2.05% 8.53	3.57 15.08	36% 44%	62% 75%	776.10 775.28	776.03 773.30	779.10 779.21	30 Out
19A		Inlet	12	35.3	0.7082	0.0668	0.047		0.047	0.18	5.00	7.45	0.35	10.42	0.62	0.50% 3.22	2.53	14%	24%	774.07	773.89	777.07	19B
19B		Inlet	12	9.1	0.7220	0.1517	0.110	0.047	0.157	0.05	5.18	7.39	1.16	10.33	2.02	0.50% 3.22	2.53	46%	80%	773.89	773.85	777.07	19
19 20A		Manhole Inlet	12	155.7 34.6	0.6851	0.0074	0.000	0.157	0.157	0.81	5.23 5.00	7.37 7.45	1.16 0.04	10.30	2.02 0.07	0.50% 3.22 1.00% 4.55	2.53 3.57	46% 1%	80% 2%	773.85 777.35	773.07 777.01	777.69 780.35	20 20B
20B		Inlet	12	27.4	0.6791	0.0090	0.006	0.005	0.003	0.10	5.13	7.41	0.08	10.35	0.14	1.00% 4.55	3.57	2%	4%	777.01	776.73	780.18	20
23A		Inlet	12	16.7	0.7541	0.0735	0.055		0.055	0.09	5.00	7.45	0.41	10.42	0.72	0.50% 3.22	2.53	16%	29%	786.74	786.66	789.74	23B
23B 24A		Inlet	12	90.9	0.7397	0.1670 0.0506	0.124	0.212	0.336	0.18	5.09	7.42 7.45	2.49 0.29	10.38	4.36 0.51	3.30% 8.26 1.00% 4.55	6.49 3.57	38% 8%	67% 14%	786.66 783.79	783.66 783.66	789.73 786.79	24B 24B
24B		Inlet	12	91.4	0.7396	0.1720	0.127	0.375	0.502	0.16	5.27	7.36	3.69	10.28	6.45	4.30% 9.43	7.41	50%	87%	783.66	779.73	786.78	25B
25A		Inlet	12	16.7	0.7529	0.0797	0.060		0.060	0.06	5.00	7.45	0.45	10.42	0.78	1.00% 4.55	3.57	13%	22%	779.82	779.73	782.82	25B
25B 20		Inlet Manhole	12	48.2 57.9	0.7385	0.1943	0.143	0.562	0.705 0.716	0.09	5.43	7.30	5.15 5.21	10.20	8.99 9.09	4.30% 9.43 0.50% 4.21	7.41 7.45	70% 70%	121%	779.73 773.07	777.66 772.78	782.81 780.96	20
21A		Inlet	12	44.6	0.7581	0.0761	0.058	0.710	0.058	0.16	5.00	7.45	0.43	10.42	0.75	1.00% 4.55	3.57	12%	21%	775.76	775.31	778.76	21B
21B		Inlet	12	4.3	0.6442	0.1772	0.114	0.058	0.172	0.02	5.16	7.39	1.27	10.34	2.22	1.00% 4.55	3.57	36%	62%	775.31	775.27	778.76	21
21 27		Manhole Inlet	18 12	62.9 33.3	0.5675	0.3206	0.000	0.888	0.888 0.182	0.25	5.75 5.00	7.20 7.45	6.39 1.36	10.04 10.42	11.14 2.37	0.50% 4.21 1.00% 4.55	7.45 3.57	86% 38%	150%	772.78 775.74	772.47 775.41	779.30 778.74	22
22		Manhole	21	67.5	0.3073	0.3200	0.000	1.070	1.070	0.24	5.99	7.11	7.61	9.91	13.25	0.50% 4.67	11.23	68%	118%	772.47	772.13	779.26	Out
39A		Inlet	12	43.7	0.7411	0.0785	0.058		0.058	0.23	5.00	7.45	0.43	10.42	0.76	0.50% 3.22	2.53	17%	30%	768.10	767.88	771.10	39
39		Manhole	12	135.1			0.000	0.058	0.058	0.23	5.23	7.37	0.43	10.30	0.75	4.50% 9.65	7.58	6%	10%	764.78	758.70	771.71	40
40A 40B		Inlet	12	34.6 8.0	0.6240 0.5245	0.1303 0.5688	0.081		0.081	0.13	5.00	7.45 7.45	0.61 2.22	10.42 10.42	1.06 3.89	1.00% 4.55 1.00% 4.55	3.57 3.57	17% 62%	30% 109%	758.11 757.85	757.77 757.77	761.11 761.70	40C 40C
40B 40B		Inlet	12	9.2	0.5245	0.5688	0.298	0.380	0.298	0.03	5.00	7.45	2.22	10.42	4.99	1.00% 4.55	3.57	80%	140%	757.85	757.68	761.70	400
40 WQU2		Manhole Manhole	12 12	43.3 26.0			0.000	0.442 0.442	0.442 0.442	0.07 0.10	5.46 5.53	7.29 7.27	3.22 3.21	10.18 10.14	5.63 5.61	4.50% 9.65 1.00% 4.55	7.58 3.57	43% 90%	74% 157%	756.90 754.96	754.96 754.70	761.71 758.96	WQU2 41
								0.442															
42A 42B		Inlet	12	16.4 117.8	0.7473	0.1181 0.1400	0.088	0.088	0.088	0.06	5.00 5.06	7.45 7.43	0.66 1.43	10.42	1.15 2.50	1.00% 4.55 4.50% 9.65	3.57 7.58	18% 19%	32%	757.94 754.97	757.77 749.67	760.94 760.94	42B WQU3
WQU3		Manhole	12	28.0	5.,44,	0.1400	0.000	0.192	0.192	0.10	5.26	7.36	1.42	10.28	2.47	1.00% 4.55	3.57	40%	69%	749.67	749.39	752.68	43
224			- 10	400	0.07.10	0.5122	0.255	1.070	1.000	0.05	7.0	0.71	0.53	0.00	16.53	1,0001 5.05	10.50	91%	1570/	757.00	75000		
33A 31B		Inlet	18 12	18.0 66.4	0.6548	0.5439 0.0739	0.356	1.070	1.426 0.057	0.05	7.19 5.00	6.71 7.45	9.57 0.43	9.29 10.42	16.57 0.75	1.00% 5.96 8.49% 13.25	10.53 10.41	91% 4%	157% 7%	757.08 769.09	756.84 763.45	772.09	33 32B
32B		Inlet	12	74.3	0.8200	0.0210	0.017	0.057	0.075	0.09	5.08	7.42	0.55	10.38	0.97	8.83% 13.52	10.62	5%	9%	765.90	759.34	766.45	33
33		Inlet	21	31.4	0.8200	0.0303	0.025	1.501	1.526	0.08	7.24	6.69	10.21	9.27	17.67	1.00% 6.61	15.89	64%	111%	756.84	756.45	760.13	34
31A 32A		Inlet	12	66.0	0.7183	0.6194 0.0416	0.445	0.445	0.445	0.08	5.00	7.45 7.42	3.32	10.42	5.80 6.21	8.49% 13.25 7.32% 12.31	10.41 9.66	32% 37%	56% 64%	768.51 765.36	762.91 760.78	771.51 765.91	31A 34
34		Inlet	21	49.4	0.8200	0.0416	0.034	2.005	2.039	0.08	7.31	6.66	13.59	9.23	23.52	1.00% 6.61	15.89	86%	148%	756.45	755.95	760.58	35
36A		Inlet	18	27.8	0.5840	0.2304	0.135	0.992	1.126	0.08	8.24	6.35	7.15	8.75	12.32	1.00% 5.96	10.53	68%	117%	761.50	761.22	-	35
36 WOU		Manhole Manhole	24	13.2			0.000	3.166	3.166	0.03	8.32 8.35	6.32	20.01	8.71 8.69	34.46 34.40	1.00% 7.22 1.00% 7.22	22.68	88% 88%	152% 152%	755.95	755.82	765.39	WQU4 36
WQU4		iviannoie	24	8.8	_		0.000	3.166	3.166	0.02	8.35	0.31	19.98	0.09	34.40	1.00% 7.22	22.68	00%	132%	755.82	755.73	764.57	36

Erosion-Protection Method	Velocity, v (ft/s)
Revetment Riprap	≤ 6.5
Class 1 Riprap	6.5 < v < 10
Class 2 Riprap	$10 \le v \le 13$
Energy Dissipator	> 13

Note: If clear-zone or other issues prohibit the use of the required erosion-protection method, the Office of Hydraulics should be contacted for additional instructions.

STREAM VELOCITY FOR EROSION PROTECTION

Figure 203-2D